BURNS COOLEY DENNIS, INC.

GEOTECHNICAL AND MATERIALS ENGINEERING CONSULTANTS

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August 2, 2017

Walker Circle Properties, LLC
Post Office Box 180789
Richland, Mississippi 39218

Project No. 170287

Attention: B.T. Steadman

Soils Data Report
Truck Parking Facility
Richland, Mississippi

Gentlemen:

Submitted here is the report of our soil data report for the above-captioned project. This investigation was authorized by Mr. B.T. Steadman on May 17, 2017. The investigation was generally performed in accordance with proposal sent by email on May 10, 2017.

Introduction

Plans are being made for the construction of a new truck parking facility on two parcels of land located at the southwest corner of the intersection of Leggett Drive and Old Highway 49 North in Richland, Mississippi. The facility would consist of two buildings and either asphalt or concrete pavements. Detail construction plans are not available at this time. A site plan showing the approximate locations of the buildings and the boring locations is presented on Figure 1 of this report. The scope of this study includes only obtaining soil data for the site; detailed geotechnical recommendations are not included as part of this study.

Field Exploration

Subsurface soil conditions were explored by means of 15 borings. The boring locations and depths were determined by a Burns Cooley Dennis representative. The approximate locations of the borings are shown on Figure 1. The borings were located in the field by the use of a hand-held GPS unit with coordinates depicted from Google Earth. Approximate GPS coordinates for the boring locations are shown at the bottom of the graphical logs within the "Comments" section.

All soils were classified in general accordance with the Unified Soil Classification System. A synopsis of the Unified Soil Classification System is presented on Figure 2 along with symbols and terminology typically utilized on graphical soil boring logs. Graphical logs of the borings are presented on Figures 3 through 17. The graphical logs illustrate the types of soil and stratification encountered with depth below the existing ground surface at the individual boring locations.

Borings 1 through 15 were made to exploration depths of 6 ft and 10 ft. Borings 1 through 7 and 9 through 11 were made using a buggy-mounted drill rig. The remaining borings were made using a hand auger due to access restrictions for the drill rig. Representative disturbed samples of the soils encountered during drilling were taken directly from the auger cuttings at an approximate 2-ft to 3-ft intervals of depth. The depths at which the auger cutting samples were taken are illustrated as small I-shaped symbols under the "Samples" column of the graphic boring logs.

All soils encountered during drilling were examined and classified in the field by a geotechnical engineering technician. The auger cutting samples were sealed in jars to provide material for visual examination and testing in the laboratory. Unless other disposition is requested, we routinely discard soil samples after about six months of storage.

Observations were made continuously during auger drilling to detect free water entering the open boreholes. Notes pertaining to groundwater observations are included at the bottom right corner of the graphic boring logs. The boreholes were backfilled with bentonite-cement grout after completion of drilling and sampling. It should be noted that at the time of our initial attempt at field investigation, the site had standing water on the order of about 2 ft to 3 ft covering about 3/4th of the site. We had to allow the site to dry before we could complete the field investigation.

Laboratory Testing

All of the soil samples were examined in the laboratory and tests were performed on select samples to assist in evaluating the strengths, classifications and volume change properties of the soils encountered. The types of laboratory tests performed are described in the following paragraphs.

Classification Tests. The classifications and volume change properties of fine-grained soils were investigated by means of Atterberg liquid and plastic limit tests. The results of the liquid and plastic limit tests are plotted as small crosses interconnected by dashed lines in the data section of the graphic boring logs. In accordance with the Unified Soil Classification System, fine-grained soils are classified as either clays or silts of low or high plasticity based on the results of Atterberg limit tests. The numerical difference between the liquid limit and plastic limit is defined as the plasticity index (PI). The magnitudes of the liquid limit and plasticity index and the proximity of the natural water content to the plastic limit are indicators of the potential for a fine-grained soil to shrink or swell upon changes in moisture content or to

consolidate under loading. The proximity of the natural water content to the plastic limit is also an indicator of soil strength.

Water Content Tests. Water content tests were performed on samples to corroborate field classifications and to extend the usefulness of the plasticity data. The results of the water content tests are plotted as small shaded circles in the data section of the graphic boring logs. The water content data have been interconnected on the logs to illustrate a continuous profile with depth.

General Subsurface Conditions

A general description of subsurface soils and groundwater conditions revealed by the borings made for this investigation is provided in the following paragraphs. The graphical logs shown on Figures 3 through 17 should be referred to for specific soil and groundwater conditions encountered at each boring location. Stick logs of the borings are shown in profile on Figures 18 and 19 to aid in visualizing subsurface soil conditions. Tabulated adjacent to the stick logs are liquid and plastic limits and water contents.

The ground surface at the boring locations was found to be underlain by sandy and silty clays (CL) to depths ranging from 2 ft to 6-ft or 10-ft completion depths of the borings. The sandy and silty clays (CL) encountered within the borings are classified as soft to very stiff with respect to consistency and are considered to have low to high strength, low to high compressibility, and low shrink/swell potential.

The silty clays (CL) are underlain by clays (CH) in Borings 1, 4, 5, 7, 9, 10, 11, and 13. The clays (CH) extend to various depths including the 6-ft and 10-ft completion depths in some borings. The clays (CH) are classified as medium stiff to very stiff with respect to consistency and considered to have moderate to high strength, low to moderate compressibility, and have moderate to high shrink/swell potential.

The silty clays (CL) and clays (CH) are underlain by silty sands (SM) or clayey sands (SC) in Borings 4, 6, and 15. The silty and clayey sands (SM,SC) are characterized as medium dense with respect to density and are considered to have moderate-high strength, low compressibility, and are nonexpansive.

Groundwater. Free water was encountered within Borings 4, 6, and 12 at depths ranging from about 3.5 ft to 9 ft. After a brief 15 minute waiting period the water levels were recorded at depths ranging from 2 ft to 8.33 ft. It should be noted that at the time of our initial field investigation on June 30, 2017 that about 3/4th of the site had about 2 ft to 3 ft of standing water on it and it was not until July 20, 2017 that the site had dried up enough to complete drilling operations. In our opinion, groundwater conditions at the site will be influenced by rainfall, surface drainage, and by the rise and fall of water levels in nearby ditches, creeks, ponds or other bodies of water. Groundwater conditions at the site can also be influenced by man-made changes. Surficial soils can become saturated and weak to relatively shallow depths during periods of prolonged and heavy rainfall.

Discussion

Subsurface soils encountered within the maximum 10-ft exploration depth of the borings made for this investigation generally consist of silty clays (CL), sands (SC and SM) and clays (CH) with lessor amounts of sandy clays (CL). The sands (SC and SM) have no potential for shrinking and swelling. The silty clays (CL) and sandy clays (CL) are considered to have low shrink/swell potential. The clays (CH) are expansive with moderate to high shrink/swell potential.

The weaker soft to medium stiff silty clays (CL) and clays (CH) that were encountered are considered to provide poor subgrade support strength for pavements and foundations. The remaining subsurface soils encountered in the borings are considered to be stronger and less compressible. The stronger silty clays (CL), sandy clays (CL) and sands (SC and SM) would provide good subgrade support strength for pavements and foundations. The clays (CH) would not provide adequate long term subgrade support for pavements and/or foundations due to their potential to shrinking and swelling due to potential changes in moisture content.

Shrink/swell movements associated with seasonal moisture content fluctuations can occur within the expansive clay (CH) soils that were encountered. Cover soils overlying expansive clays (CH) buffer moisture content changes within the clays (CH) caused by seasonal weather conditions and transpiration by plants and trees. Thus, the potential magnitude of moisture content changes and associated shrink/swell movements within expansive clays (CH) is proportionate to the thickness of overlying cover soils. Moisture content changes and shrink/swell movements within expansive clays (CH) are lower for greater thicknesses of cover soils. There is a general trend for expansive clays (CH) beneath pavements and/or foundations to swell due to an increase in water content caused by capillary and vapor phase movement of moisture within the clays (CH). Expansive clays (CH) will also experience considerable swelling if directly supplied with water from rainfall, sprinkler systems, broken underground water and sewer pipes, or any other source. Trees growing adjacent to pavements and/or foundations can extract a considerable amount of moisture from the ground, resulting in localized shrinkage of expansive clays (CH) accompanied by vertical and lateral movements. Overburden removal associated with the establishment of finished grades lower than existing ground surface elevations will cause stress relief in expansive clays (CH) resulting in long-term rebound. Expansive clay (CH) soils will also experience long-term downhill creep movements, depending on slope steepness.

Typically in this region when a site contains moderate to highly expansive clays (CH), building can be supported by a shallow foundation system and pavements can consist of either asphalt concrete or rigid Portland cement concrete (PCC) provided proper earthwork procedures are followed and there exists a low permeability and low shrink/swell potential soil buffer zone directly beneath foundations and pavements. Typically these buffers are 5 ft for structures and 3 ft for pavements.

Report Limitations

We caution that this report merely presents subsurface conditions encountered at the locations of the borings. It should be understood that subsurface conditions between and beyond the borings might differ from those encountered at the boring locations. The only warranty made by us in connection with the services provided is we have used that degree of care and skill ordinarily exercised under similar conditions by reputable members of our profession practicing in the same or similar locality. No other warranty, express or implied, is made or intended.

We appreciate the opportunity to be of service. If you should have any questions concerning this report, please do not hesitate to call us.

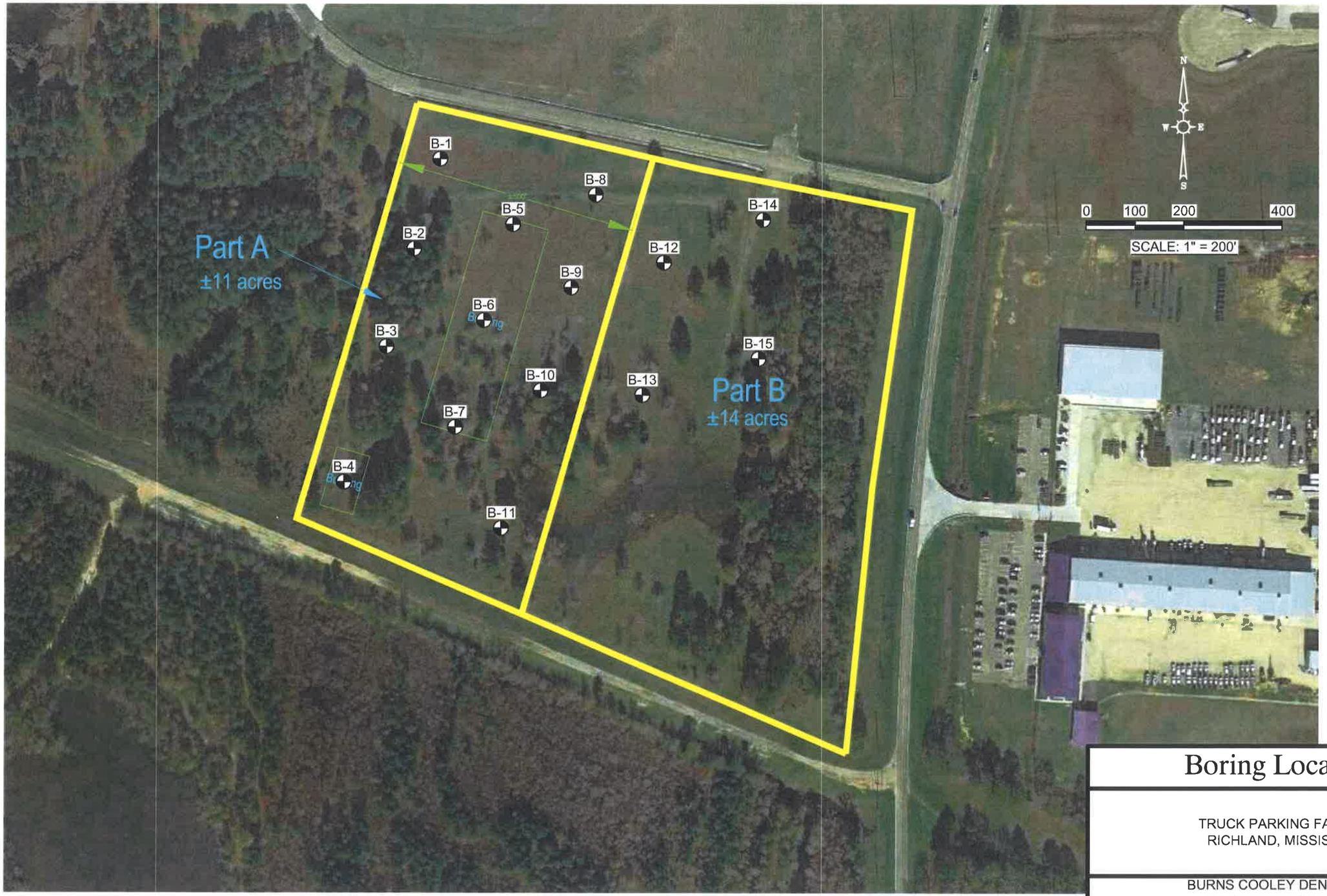
Very truly yours,

BURNS COOLEY DENNIS, INC.

Al Marioni, E. I.

G. Thomas Dunlap, P.E.

APM/GTD/apm
Copies Submitted: Via electronic mail



Boring Locations

TRUCK PARKING FACILITY
RICHLAND, MISSISSIPPI

BURNS COOLEY DENNIS, INC.
551 SUNNYBROOK ROAD
RIDGELAND, MISSISSIPPI 39157

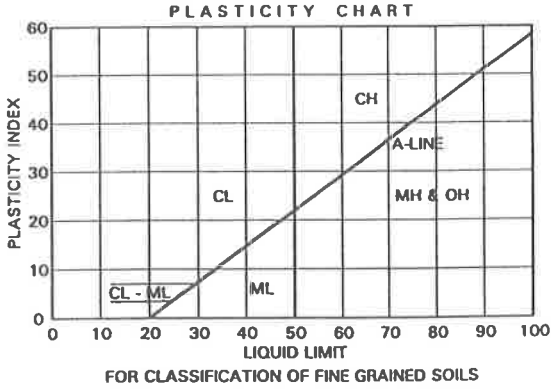
JOB NO. 170287	SCALE: AS SHOWN	FIGURE 1

UNIFIED SOIL CLASSIFICATION SYSTEM

MAJOR DIVISIONS			SYMBOL & LETTER	DESCRIPTION
COARSE-GRAINED SOILS More than half of material larger than No. 200 sieve size	GRAVELS More than half of coarse fraction larger than No. 4 sieve size	Clean Gravels (Little or no fines)	GW	WELL GRADED GRAVEL, GRAVEL-SAND MIXTURE
			GP	POORLY GRADED GRAVEL, GRAVEL-SAND MIXTURE
		Gravels with fines (Appreciable amount of fines)	GM	SILTY GRAVEL, GRAVEL-SAND-SILT MIXTURE
			GC	CLAYEY GRAVEL, GRAVEL-SAND-CLAY MIXTURE
	SANDS More than half of coarse fraction smaller than No. 4 sieve size	Clean Sands (Little or no fines)	SW	WELL GRADED SAND, GRAVELLY SAND
			SP	POORLY GRADED SAND, GRAVELLY SAND
		Sands with fines (Appreciable amount of fines)	SM	SILTY SAND, SAND-SILT MIXTURE
			SC	CLAYEY SAND, SAND-CLAY MIXTURE
FINE-GRAINED SOILS More than half of material smaller than No. 200 sieve	SILTS AND CLAYS	Liquid limit less than 50	ML	SILT WITH LITTLE OR NO PLASTICITY
			ML	CLAYEY SILT, SILT WITH SLIGHT TO MEDIUM PLASTICITY
			CL	SILTY CLAY, LOW TO MEDIUM PLASTICITY
			CL	SANDY CLAY, LOW TO MEDIUM PLASTICITY (30% TO 50% SAND)
	SILTS AND CLAYS	Liquid limit greater than 50	MH	SILT, FINE SANDY OR SILTY SOIL WITH HIGH PLASTICITY
			CH	CLAY, HIGH PLASTICITY
			OH	ORGANIC CLAY OF MEDIUM TO HIGH PLASTICITY
HIGHLY ORGANIC SOILS			PT	PEAT, HUMUS, SWAMP SOIL

TERMS CHARACTERIZING SOIL STRUCTURE

Slickensided	- Clays with polished and striated planes created as a result of volume changes related to shrinking, swelling and/or changes in overburden pressure.
Fissured	- Clays with a blocky or jointed structure generally created by seasonal shrinking and swelling.
Laminated	- Composed of thin alternating layers of varying color and texture.
Calcareous	- Containing appreciable quantities of calcium carbonate.
Parting	- Paper thin (less than 1/8 inch).
Seam	- 1/8 inch to 3 inch thickness.
Layer	- Greater than 3 inches in thickness.



FOR CLASSIFICATION OF FINE GRAINED SOILS

DENSITY AND CONSISTENCY

COARSE-GRAINED SOILS		FINE-GRAINED SOILS		
DENSITY	PENETRATION RESISTANCE, N Blows per Foot	CONSISTENCY	COHESION Kips/Sq.Ft	PENETRATION RESISTANCE, N Blows per Foot
Very loose	0 - 4	Very Soft	<0.25	0 - 1
Loose	5 - 10	Soft	0.25 - 0.50	2 - 4
Medium Dense	11 - 30	Medium Stiff	0.50 - 1.00	5 - 8
Dense	31 - 50	Stiff	1.00 - 2.00	9 - 15
Very Dense	>50	Very Stiff	2.00 - 4.00	16 - 30
		Hard	>4.00	>30

PARTICLE SIZE IDENTIFICATION

Cobbles	- Greater than 3 inches
Gravel	- Coarse - 3/4 inch to 3 inches Fine - 4.76 mm to 3/4 inch
Sand	- Coarse - 2 mm to 4.76mm Medium - 0.42 mm to 2 mm Fine - 0.074 mm to 0.42 mm
Silt & Clay	- Less than 0.074 mm

RELATIVE COMPOSITION

Slightly	5 - 15%
With	16 - 29%
Sandy	30 - 50%
(or gravelly)	

SAMPLE TYPES
(Shown in Sample Column)


Shelby Tube

Split Spoon

No Recovery

Auger

Dennison Barrell

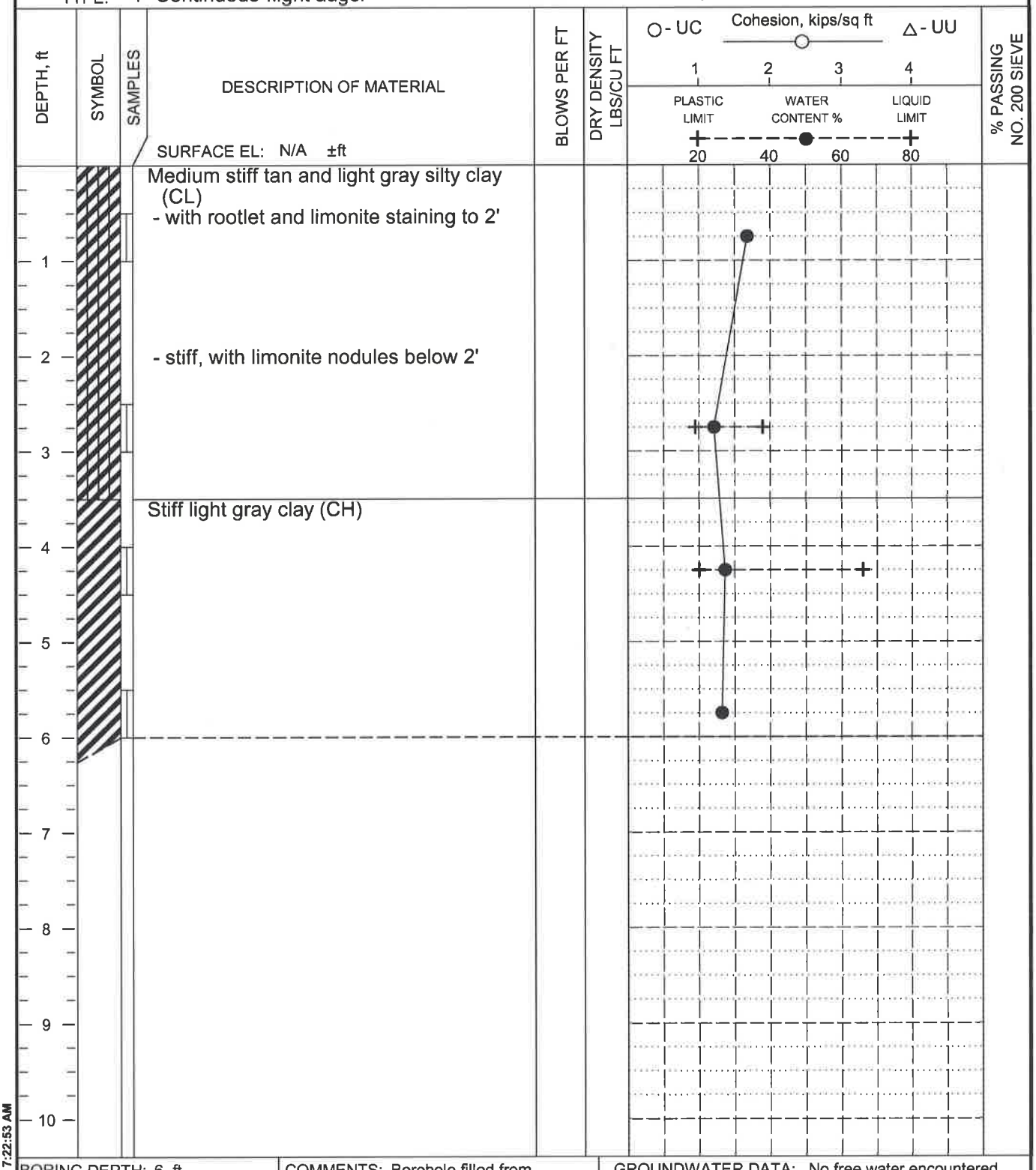
CLASSIFICATION, SYMBOLS AND TERMS USED ON GRAPHICAL BORING LOGS

LOG OF BORING NO. 1

TRUCK PARKING FACILITY
RICHLAND, MISSISSIPPI

TYPE: 4" Continuous-flight auger

LOCATION: See Figure 1



BORING DEPTH: 6 ft

DATE: 07/20/17

COMMENTS: Borehole filled from bottom-to-top with cement grout upon completion of drilling and sampling.
GPS Coordinates
N 32° 15' 41.0"
W 90° 10' 34.7"

GROUNDWATER DATA: No free water encountered during auger drilling.

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FIGURE 3

LOG OF BORING NO. 2

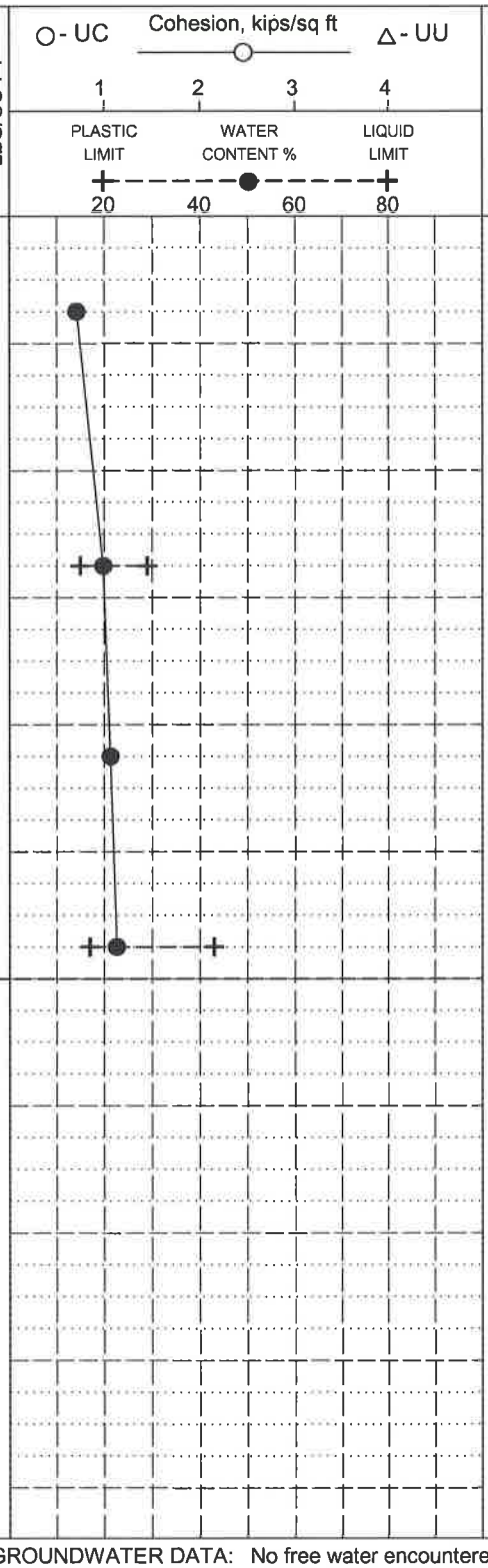
TRUCK PARKING FACILITY
RICHLAND, MISSISSIPPI

TYPE: 4" Continuous-flight auger

LOCATION: See Figure 1

SURFACE EL: N/A ±ft

DEPTH, ft	SYMBOL	SAMPLES	DESCRIPTION OF MATERIAL	BLOWS PER FT	DRY DENSITY LBS/CU FT	% PASSING NO. 200 SIEVE
0			Stiff tan and light gray silty clay (CL) - with sand to 4'			
1						
2			- medium stiff 2'-4'			
3						
4						
5						
6						
7						
8						
9						
10						



BORING DEPTH: 6 ft

DATE: 07/20/17

COMMENTS: Borehole filled from bottom-to-top with cement grout upon completion of drilling and sampling.
GPS Coordinates
N 32° 15' 39.2"
W 90° 10' 35.4"

GROUNDWATER DATA: No free water encountered during auger drilling.

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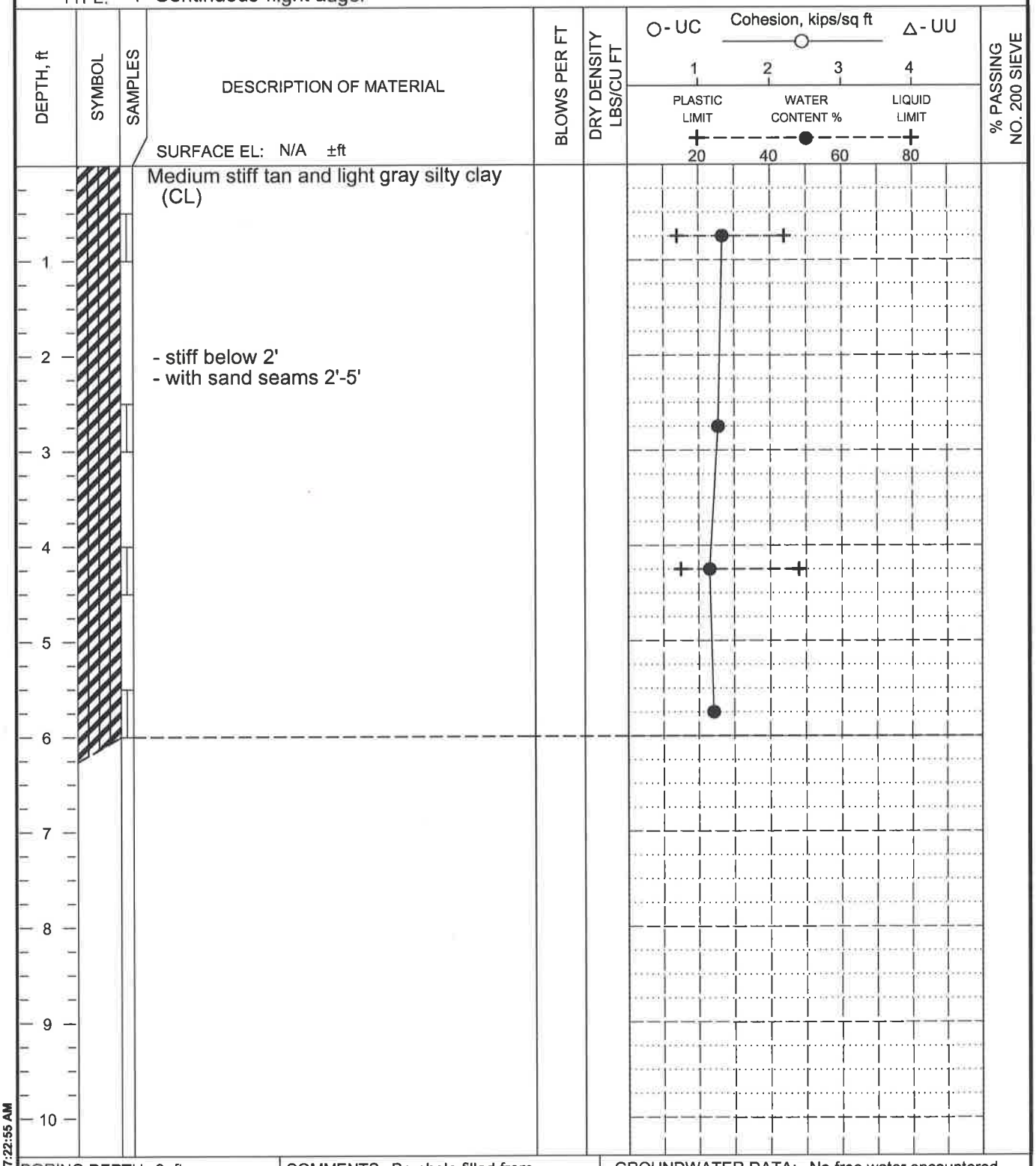
FIGURE 4

LOG OF BORING NO. 3

TRUCK PARKING FACILITY
RICHLAND, MISSISSIPPI

TYPE: 4" Continuous-flight auger

LOCATION: See Figure 1



BORING DEPTH: 6 ft

DATE: 07/20/17

COMMENTS: Borehole filled from bottom-to-top with cement grout upon completion of drilling and sampling.
GPS Coordinates
N 32° 15' 37.3"
W 90° 10' 36.0"

GROUNDWATER DATA: No free water encountered during auger drilling.

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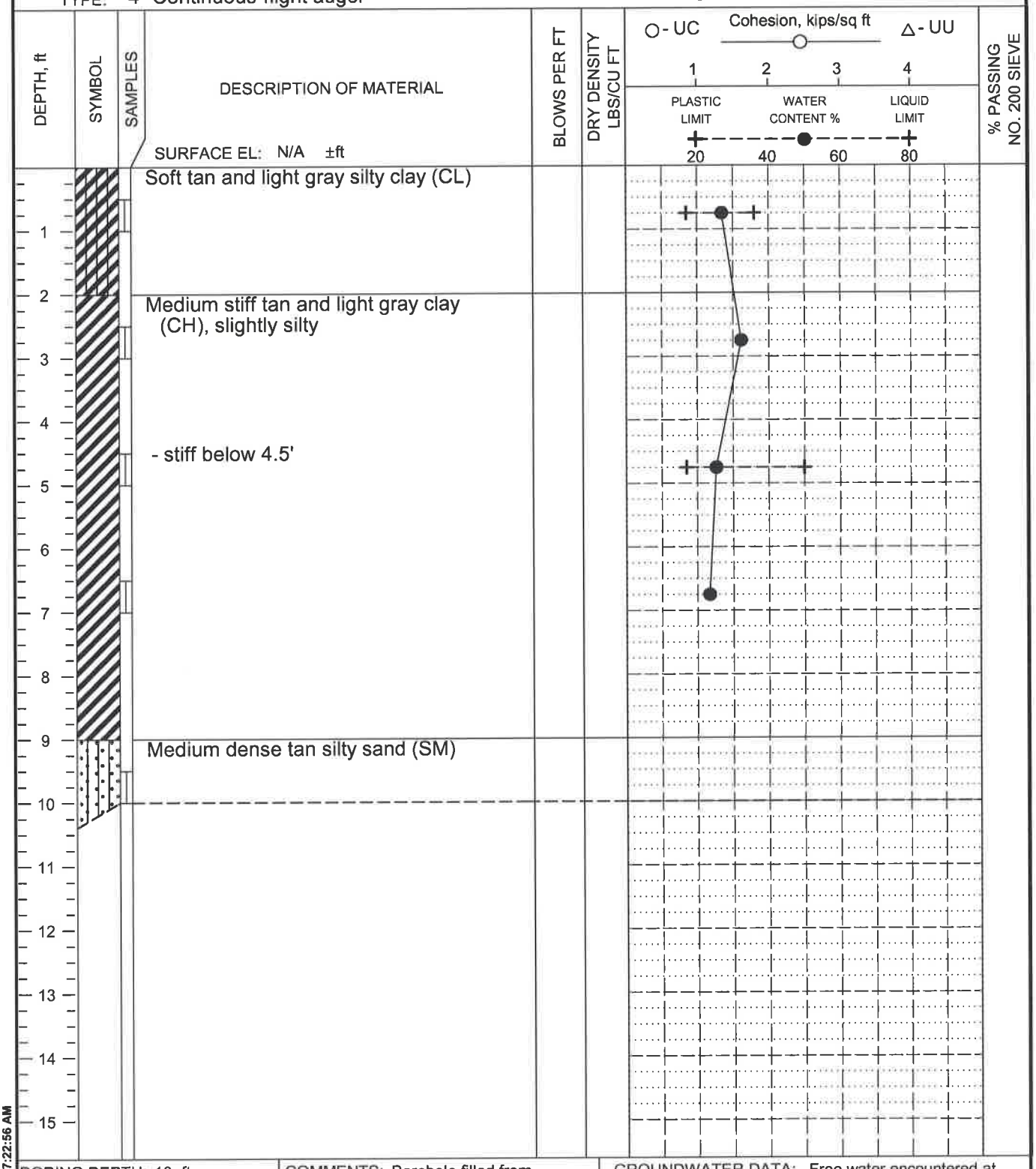
FIGURE 5

LOG OF BORING NO. 4

TRUCK PARKING FACILITY
RICHLAND, MISSISSIPPI

TYPE: 4" Continuous-flight auger

LOCATION: See Figure 1



BORING DEPTH: 10 ft

DATE: 07/20/17

COMMENTS: Borehole filled from bottom-to-top with cement grout upon completion of drilling and sampling.
GPS Coordinates
N 32° 15' 34.6"
W 90° 10' 37.0"

GROUNDWATER DATA: Free water encountered at an approximate depth of 9' during auger drilling. Water level at an approximate depth of 8.33' after about 15 minutes.

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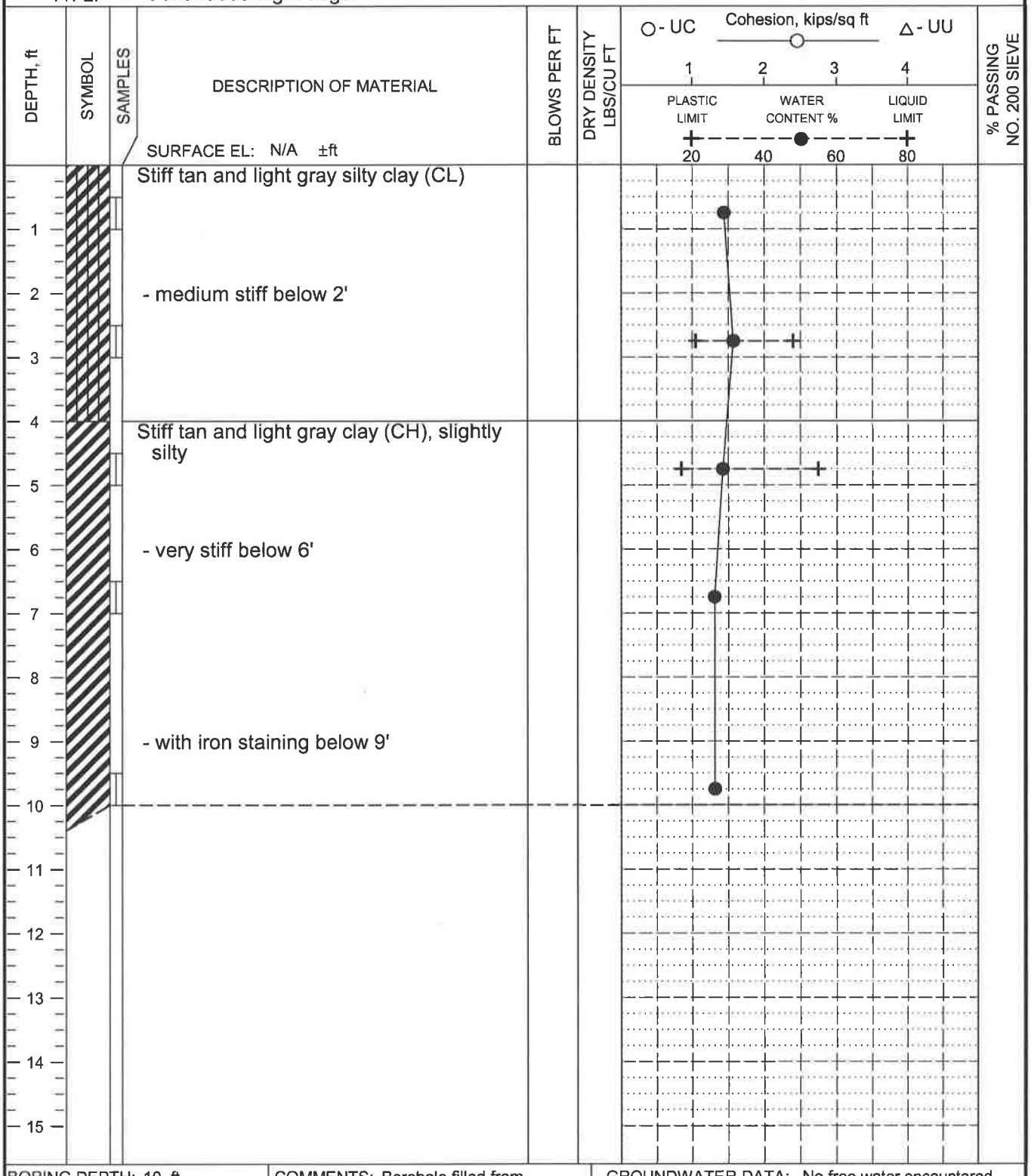
FIGURE 6

LOG OF BORING NO. 5

TRUCK PARKING FACILITY
RICHLAND, MISSISSIPPI

TYPE: 4" Continuous-flight auger

LOCATION: See Figure 1



BORING DEPTH: 10 ft

DATE: 07/20/17

COMMENTS: Borehole filled from bottom-to-top with cement grout upon completion of drilling and sampling.
GPS Coordinates
N 32° 15' 39.8"
W 90° 10' 38.1"

GROUNDWATER DATA: No free water encountered during auger drilling.

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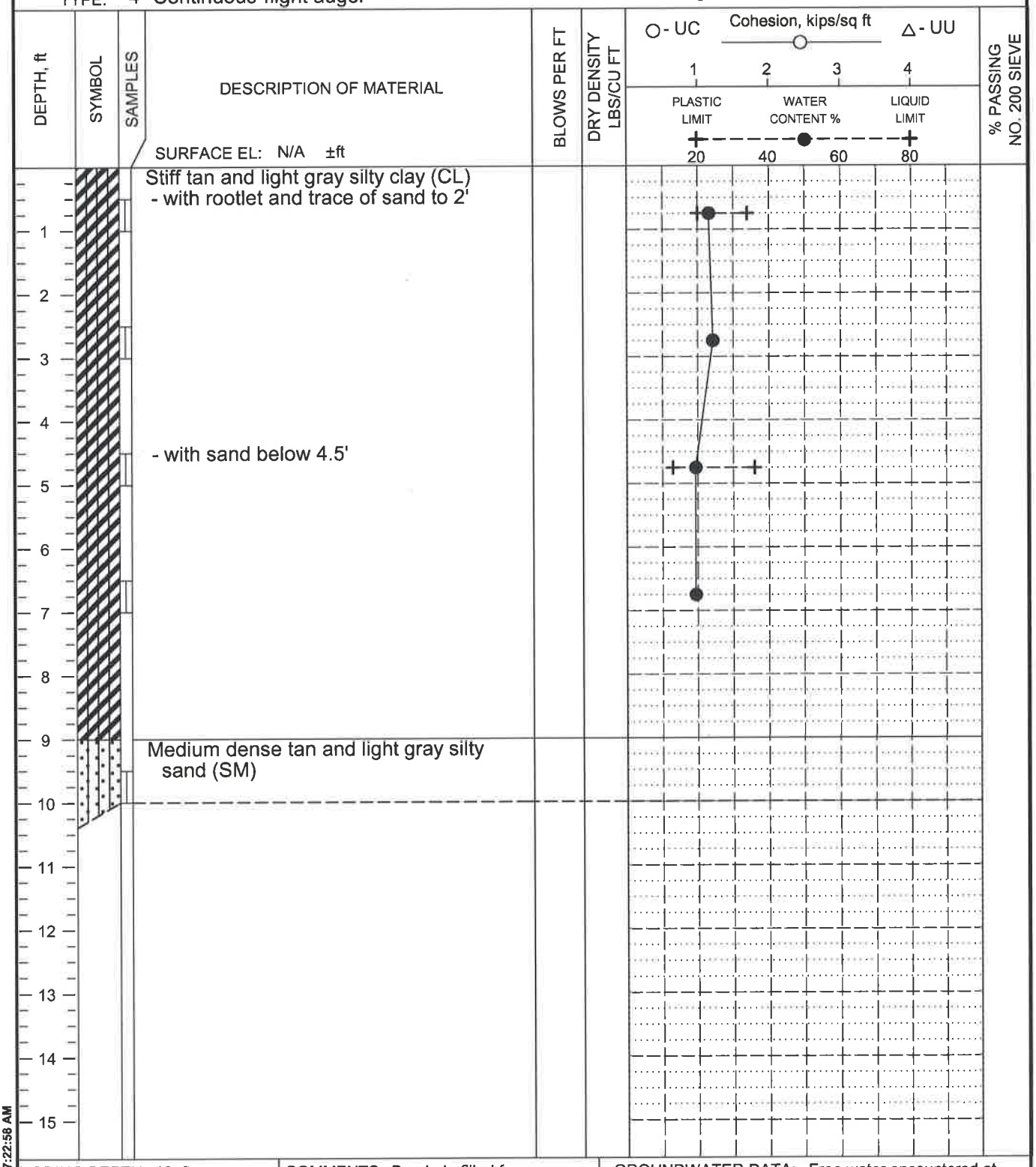
FIGURE 7

LOG OF BORING NO. 6

TRUCK PARKING FACILITY
RICHLAND, MISSISSIPPI

TYPE: 4" Continuous-flight auger

LOCATION: See Figure 1



BORING DEPTH: 10 ft

DATE: 07/20/17

COMMENTS: Borehole filled from bottom-to-top with cement grout upon completion of drilling and sampling.
GPS Coordinates
N 32° 15' 37.9"
W 90° 10' 33.7"

GROUNDWATER DATA: Free water encountered at an approximate depth of 9' during auger drilling. Water level at an approximate depth of 8' after about 15 minutes.

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FIGURE 8

LOG OF BORING NO. 7

TRUCK PARKING FACILITY
RICHLAND, MISSISSIPPI

TYPE: 4" Continuous-flight auger LOCATION: See Figure 1

DEPTH, ft	SYMBOL	SAMPLES	DESCRIPTION OF MATERIAL	BLOWS PER FT	DRY DENSITY LBS/CU FT	Cohesion, kips/sq ft (○- UC, △- UU) / PLASTIC LIMIT, WATER CONTENT %, LIQUID LIMIT	% PASSING NO. 200 SIEVE
			SURFACE EL: N/A ±ft				
0			Stiff tan and light gray silty clay (CL) - with rootlet and limonite staining to 2'				
1							
2							
3							
4			- with limonite nodules below 4.5'				
5							
6			Stiff tan and light gray clay (CH), slightly silty				
7							
8							
9			- light gray, with limonite nodules below 9'				
10							
11							
12							
13							
14							
15							

Cohesion, kips/sq ft: 1, 2, 3, 4
PLASTIC LIMIT / WATER CONTENT % / LIQUID LIMIT: 20, 40, 60, 80

BORING DEPTH: 10 ft

DATE: 07/20/17

COMMENTS: Borehole filled from bottom-to-top with cement grout upon completion of drilling and sampling.
GPS Coordinates
N 32° 15' 35.7"
W 90° 10' 34.4"

GROUNDWATER DATA: No free water encountered during auger drilling.

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FIGURE 9

LOG OF BORING NO. 8

TRUCK PARKING FACILITY
RICHLAND, MISSISSIPPI

TYPE: 3" Hand auger

LOCATION: See Figure 1

SURFACE EL: N/A ±ft

DEPTH, ft	SYMBOL	SAMPLES	DESCRIPTION OF MATERIAL	BLOWS PER FT	DRY DENSITY LBS/CU FT	% PASSING NO. 200 SIEVE
0			Stiff tan silty clay (CL) - with sand and rootlet to 2'			
2			- tan and light gray below 2' - with limonite nodules and staining 2'-5'			
5			- very stiff below 5'			

O - UC Cohesion, kips/sq ft Δ - UU

1 2 3 4

PLASTIC LIMIT WATER CONTENT % LIQUID LIMIT

20 40 60 80

BORING DEPTH: 6 ft

DATE: 06/30/17

COMMENTS: Borehole filled from bottom-to-top with cement grout upon completion of drilling and sampling.
GPS Coordinates
N 32° 15' 40.32"
W 90° 10' 31.0"

GROUNDWATER DATA: No free water encountered during auger drilling.

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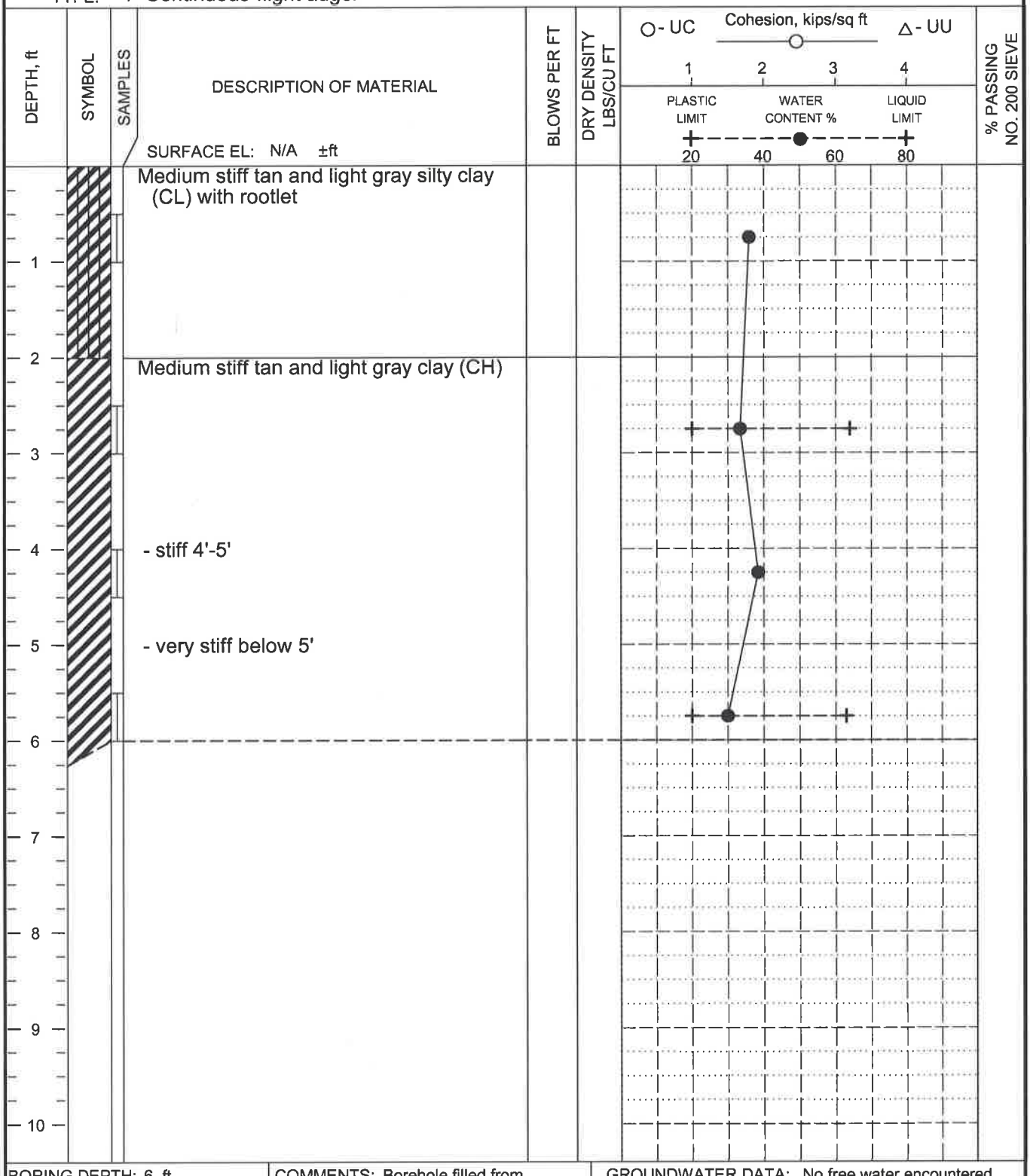
FIGURE 10

LOG OF BORING NO. 9

TRUCK PARKING FACILITY
RICHLAND, MISSISSIPPI

TYPE: 4" Continuous-flight auger

LOCATION: See Figure 1



BORING DEPTH: 6 ft

DATE: 07/20/17

COMMENTS: Borehole filled from bottom-to-top with cement grout upon completion of drilling and sampling.
GPS Coordinates
N 32° 15' 38.5"
W 90° 10' 31.7"

GROUNDWATER DATA: No free water encountered during auger drilling.

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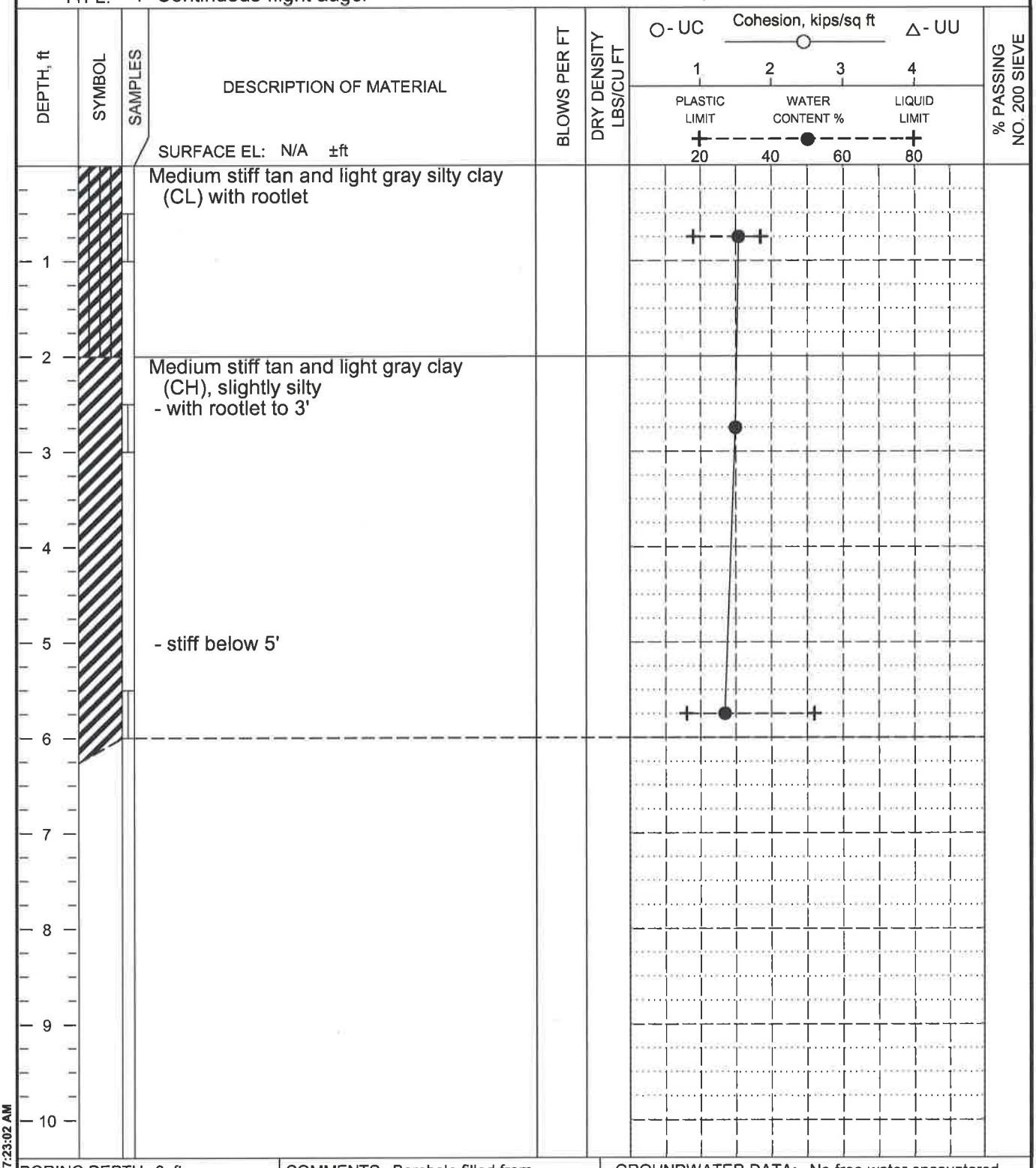
FIGURE 11

LOG OF BORING NO. 10

TRUCK PARKING FACILITY
RICHLAND, MISSISSIPPI

TYPE: 4" Continuous-flight auger

LOCATION: See Figure 1



BORING DEPTH: 6 ft

DATE: 07/20/17

COMMENTS: Borehole filled from bottom-to-top with cement grout upon completion of drilling and sampling.
GPS Coordinates
N 32° 15' 36.3"
W 90° 10' 32.4"

GROUNDWATER DATA: No free water encountered during auger drilling.

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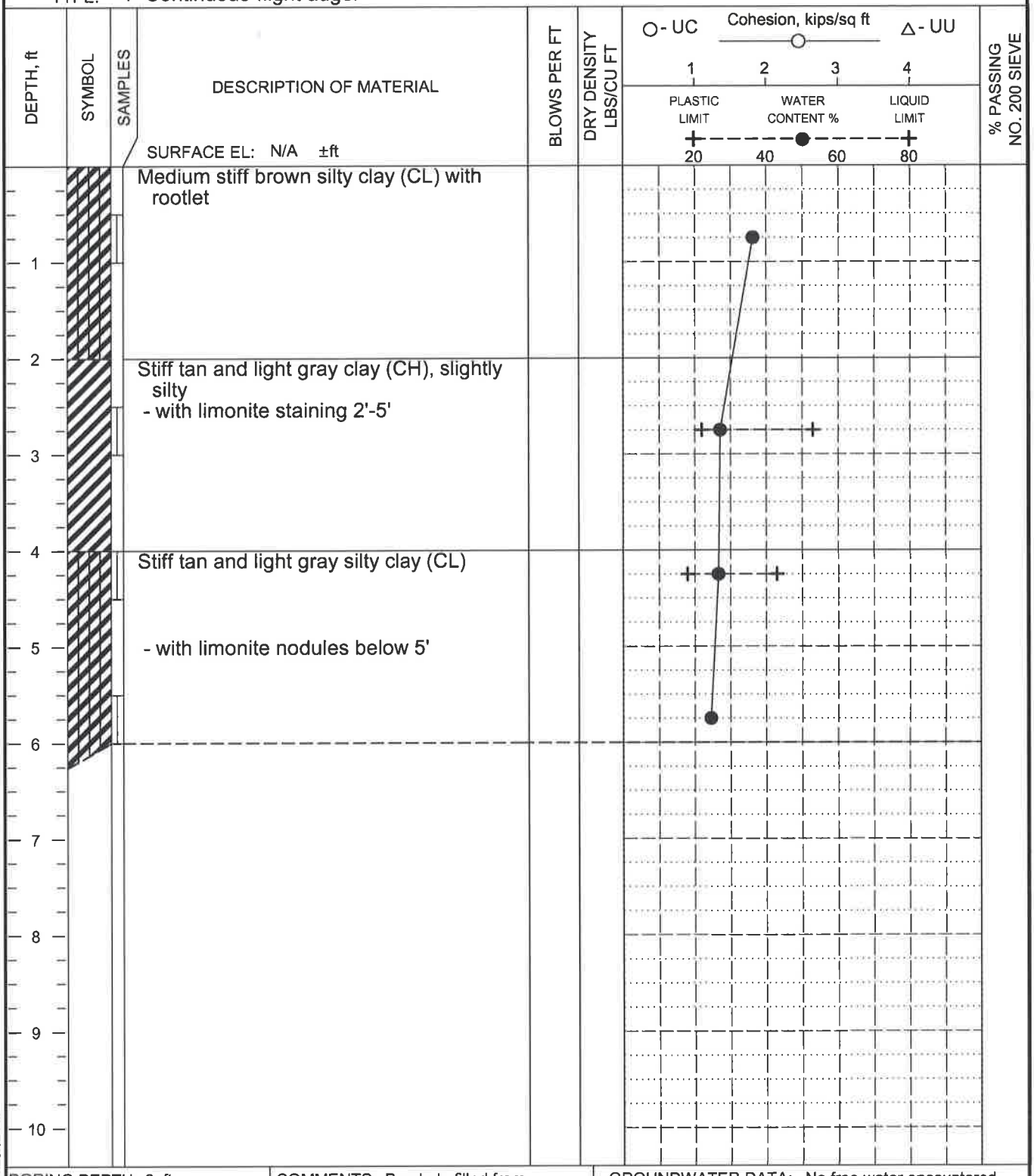
FIGURE 12

LOG OF BORING NO. 11

TRUCK PARKING FACILITY
RICHLAND, MISSISSIPPI

TYPE: 4" Continuous-flight auger

LOCATION: See Figure 1



BORING DEPTH: 6 ft

DATE: 07/20/17

COMMENTS: Borehole filled from bottom-to-top with cement grout upon completion of drilling and sampling.
GPS Coordinates
N 32° 15' 33.8"
W 90° 10' 33.3"

GROUNDWATER DATA: No free water encountered during auger drilling.

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FIGURE 13

LOG OF BORING NO. 12

TRUCK PARKING FACILITY
RICHLAND, MISSISSIPPI

TYPE: 3" Hand auger

LOCATION: See Figure 1

SURFACE EL: N/A ±ft

DEPTH, ft	SYMBOL	SAMPLES	DESCRIPTION OF MATERIAL	BLOWS PER FT	DRY DENSITY LBS/CU FT	Cohesion, kips/sq ft (○ - UC, △ - UU) 1 2 3 4 / PLASTIC LIMIT — WATER CONTENT % — LIQUID LIMIT 20 40 60 80	% PASSING NO. 200 SIEVE
0			Stiff tan and light gray sandy clay (CL) - with limonite nodules to 2'				
1							
2			- light gray and tan below 2'				
3							
4							
5			- with limonite seams below 5'				
6							
7							
8							
9							
10							

BORING DEPTH: 6 ft

DATE: 06/30/17

COMMENTS: Borehole filled from bottom-to-top with cement grout upon completion of drilling and sampling.
GPS Coordinates
N 32° 15' 39.0"
W 90° 10' 29.5"

GROUNDWATER DATA: Free water encountered at an approximate depth of 3.5' during auger drilling. Water level at an approximate depth of 2' after about 15 minutes.

170287 8/2/2017 7:23:04 AM

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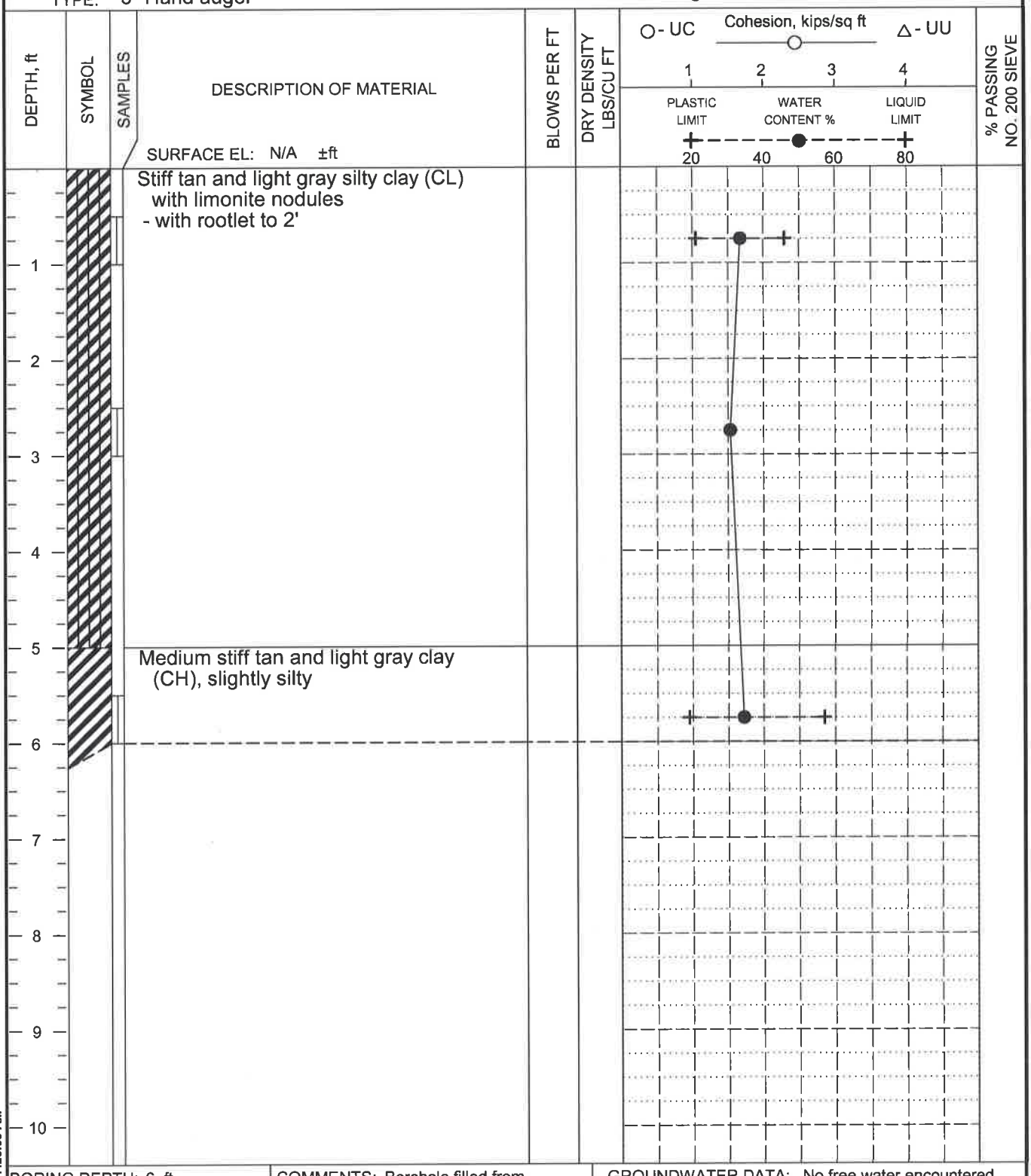
FIGURE 14

LOG OF BORING NO. 13

TRUCK PARKING FACILITY
RICHLAND, MISSISSIPPI

TYPE: 3" Hand auger

LOCATION: See Figure 1



BORING DEPTH: 6 ft

DATE: 06/30/17

COMMENTS: Borehole filled from bottom-to-top with cement grout upon completion of drilling and sampling.
GPS Coordinates
N 32° 15' 36.4"
W 90° 10' 30.0"

GROUNDWATER DATA: No free water encountered during auger drilling.

8/2/2017 7:23:05 AM
170287

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FIGURE 15

LOG OF BORING NO. 14

TRUCK PARKING FACILITY
RICHLAND, MISSISSIPPI

TYPE: 3" Hand auger

LOCATION: See Figure 1

SURFACE EL: N/A ±ft

DEPTH, ft	SYMBOL	SAMPLES	DESCRIPTION OF MATERIAL	BLOWS PER FT	DRY DENSITY LBS/CU FT	Cohesion, kips/sq ft (○ - UC, △ - UU) / PLASTIC LIMIT — WATER CONTENT % — LIQUID LIMIT	% PASSING NO. 200 SIEVE
0			Stiff tan silty clay (CL) with sand - with limonite nodules to 2'				
1							
2			- tan and light gray below 2'				
3							
4							
5			- very stiff, with limonite nodules and mica below 5'				
6							
7							
8							
9							
10							

Cohesion, kips/sq ft: 1, 2, 3, 4

PLASTIC LIMIT — WATER CONTENT % — LIQUID LIMIT: 20, 40, 60, 80

BORING DEPTH: 6 ft

DATE: 06/30/17

COMMENTS: Borehole filled from bottom-to-top with cement grout upon completion of drilling and sampling.
GPS Coordinates
N 32° 15' 40.0"
W 90° 10' 27.2"

GROUNDWATER DATA: No free water encountered during auger drilling.

170287 8/2/2017 7:23:06 AM

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FIGURE 16

LOG OF BORING NO. 15

TRUCK PARKING FACILITY
RICHLAND, MISSISSIPPI

TYPE: 3" Hand auger

LOCATION: See Figure 1

SURFACE EL: N/A ±ft

DEPTH, ft	SYMBOL	SAMPLES	DESCRIPTION OF MATERIAL	BLOWS PER FT	DRY DENSITY LBS/CU FT	Cohesion, kips/sq ft (○ - UC, △ - UU) 1 2 3 4 / PLASTIC LIMIT — WATER CONTENT % — LIQUID LIMIT 20 40 60 80	% PASSING NO. 200 SIEVE
0			Stiff tan silty clay (CL) - with trace of sand to 2'				
1							
2			- medium stiff, tan and light gray, with sand below 2'				
3							
4							
5			Medium dense light gray and tan clayey sand (SC) with roots				
6							
7							
8							
9							
10							

BORING DEPTH: 6 ft

DATE: 06/30/17

COMMENTS: Borehole filled from bottom-to-top with cement grout upon completion of drilling and sampling.
GPS Coordinates
N 32° 15' 37.1"
W 90° 10' 27.2"

GROUNDWATER DATA: No free water encountered during auger drilling.

170287 8/2/2017 7:23:07 AM

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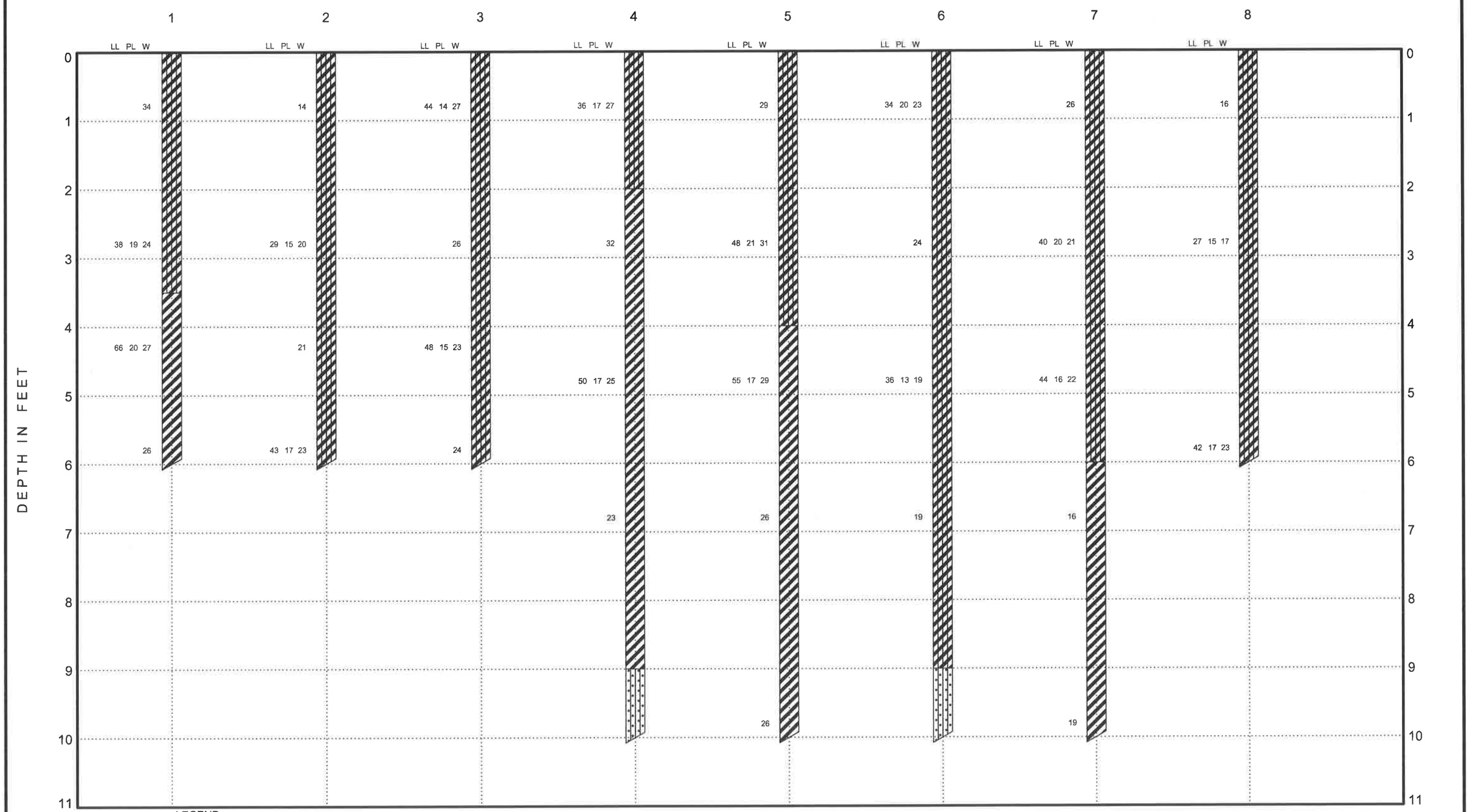
FIGURE 17



LEGEND:
LL = Liquid Limit
PL = Plastic Limit
W = Water Content
DD = Dry Density (pcf)
C/N = Cohesion (ksf)/Penetration Resistance, N (blows per ft) in the same column
% -200 = % Passing No. 200 Sieve

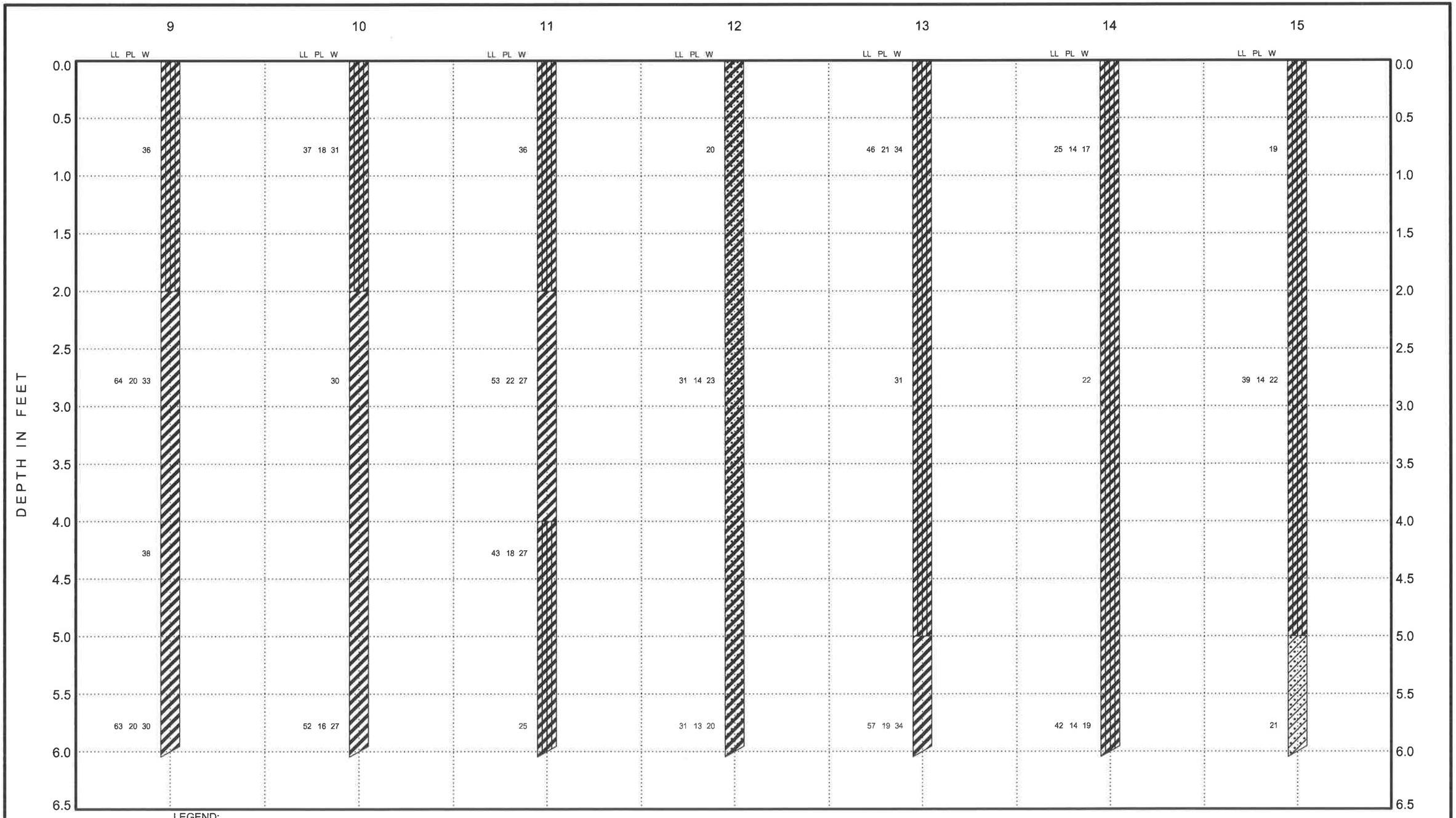
NOTE: See Figure 2 for boring log legend.

SUBSURFACE SOIL PROFILE

TRUCK PARKING FACILITY
RICHLAND, MISSISSIPPI

Job No.	170287	Date	8/2/17	Figure	18

BURNS COOLEY DENNIS, INC.



LEGEND:
LL = Liquid Limit
PL = Plastic Limit
W = Water Content
DD = Dry Density (pcf)
C/N = Cohesion (ksf)/Penetration Resistance, N (blows per ft) in the same column
% -200 = % Passing No. 200 Sieve

NOTE: See Figure 2 for boring log legend.

SUBSURFACE SOIL PROFILE

TRUCK PARKING FACILITY
RICHLAND, MISSISSIPPI

Job No.	170287	Date	8/2/17	Figure	19

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