TITLE SHEET

SITE 1 SITE LAYOUT
A1 EXTERIOR ELEVATIONS
A2 FOUNDATION PLAN
A3 MAIN BUILDING PLAN
A4 ROOM FINISH, DOOR &

WINDOW SCHEDULES, TYPICAL SECTION.

STRUCTURAL NOTES

STRUCTURAL FOUNDATION, ROOF FRAMING,& DETAILS

GENERAL NOTES

1- HVAC & ELECTRICAL WORK SHALL COMPLY WITH REQUIREMENTS OF OBBC CHAPTER 28 & 27 RESPECTIVELY AND ARE TO BE SUBMITTED BY OTHERS.

2- THE PLUMBING SYSTEM SHALL COMPLY WITH REQUIREMENTS OF OBBC CHAPTER 29 AND ARE TO BE SUBMITTED BY OTHERS.

3- FLAME SPREAD OF EXPOSED INSUL. SHALL COMPLY W/OBBC 722.2. FLAMESPREAD OF 25 OR LESS & SMOKE DEVELOPED RATING OF 450 OR LESS

29 AND ARE TO BE SUBMITTED BY OTHERS.

3- FLAME SPREAD OF EXPOSED INSUL. SHALL COMPLY W/OBBC 722.2. FLAMESPREAD OF 25 OR LESS & SMOKE DEVELOPED RATING OF 450 OR LESS.

4- FLAMESPREAD OF CONCEALED INSUL SHALL COMPLY W/OBBC 722.3. FLAMESPREAD RATING OF 75 OR LESS & SMOKE DEVELOPED RATING OF 450 OR LESS.

5- INTERIOR FINISH & TRIM SHALL BE AS FOLLOWS: CLASS II FINISHES IN EXIT ACCESS AREAS & CLASS III FINISHES IN OTHER ROOMS / AREAS.

6- CERTIFICATE OF USE & OCCUPANCY SHALL BE OBTAINED PRIOR TO THIS BUILDING BEING

OCCUPIED
7- PRIOR TO OCCUPYING THIS BUILDING BEING SHALL BE POSTED IN CONFORMANCE W/OBBC 4101:2.1.28. COORD. W/ARCHITECHT & BUILDING. DEPT.

An Office Facility For:

# National Emergency Number Association

Beecher Ridge Office Park 422 Beecher Road, Gahanna, Ohio

#### Builders

# CANINI & PELLECCHIA

Beecher Ridge Office Park 430 Beecher Road, Gahanna, Ohio 43230

## Architecture & Planning

george parker & associates, aia.

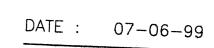
106 Short Street, Gahanna, Ohio, 43230

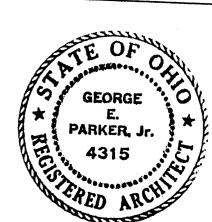
476-3600

USE GROUP TABLE

USE GROUP— 'B'
TYPE OF CONSTRUCTION— 5B—UN
FIRE RESISTANCE RATING— 2 HR
MAX LIVE LOADS— SEE ST
OCCUPANCY LOAD— GROSS
(100 SF GROSS PER OCCUPANT)

'B'
5B-UNPROTECTED
2 HR
SEE STRUCT. DWGS S1-S2
GROSS AREA = 5216 SF / 100 = 52





Architecture & Planning:

george parker & associates, a
106 Short Street, Gahanna, Ohio 43230

National Emergency Number Ass.

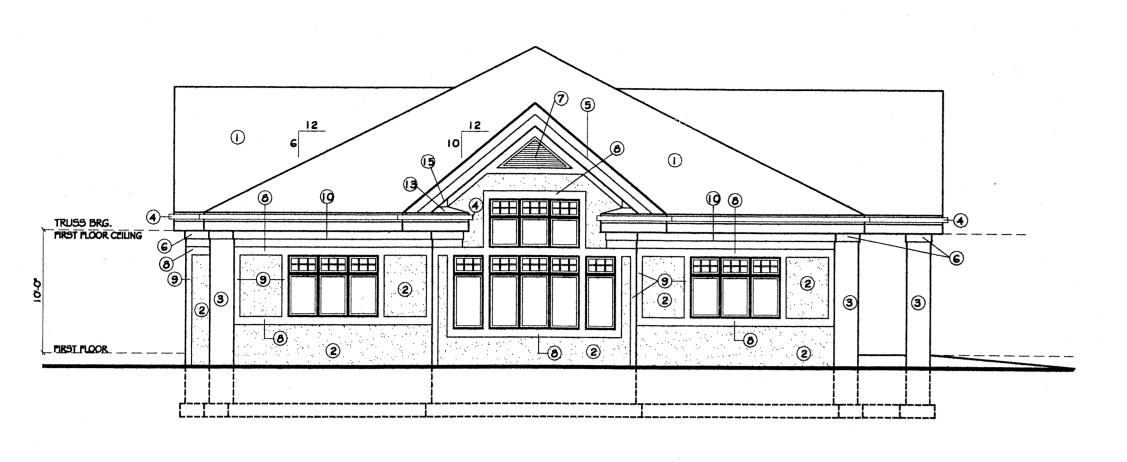
BEECHER RIDGE OFFICE PARK, GAHANNA, OHIO

DATE: 07-06-99

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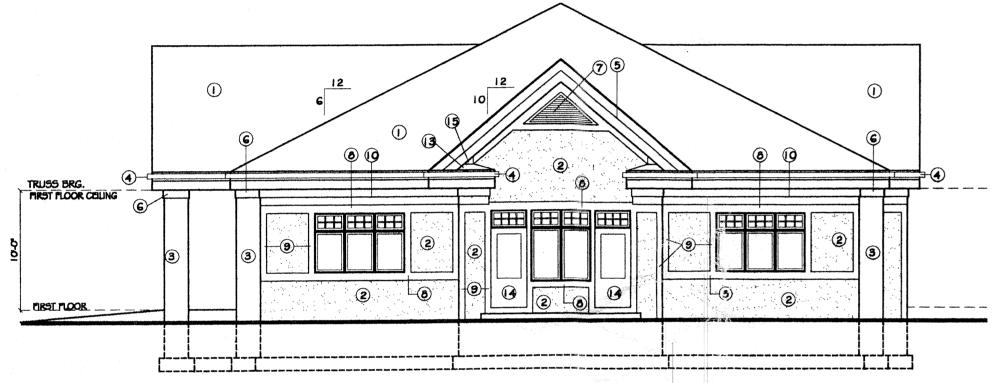
NOTE THE ENTIRE PROJECT IS NOT TO EXCEED 42000 SF.





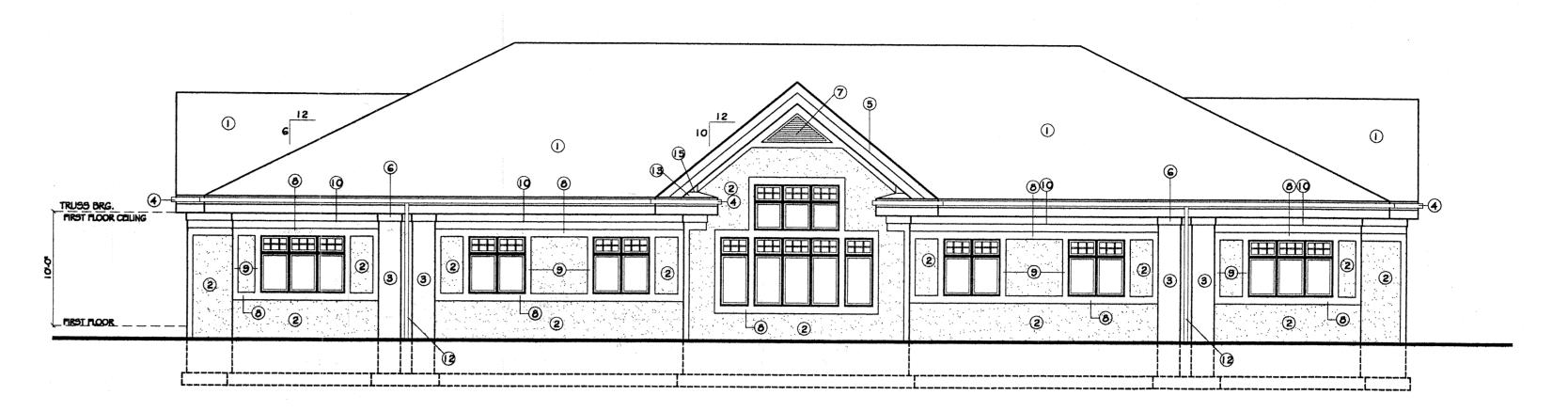
# SOUTH ELEVATION

Scale 1/8" = 1'-0"



### NORTH ELEVATION

Scale 1/8" = 1'-0"



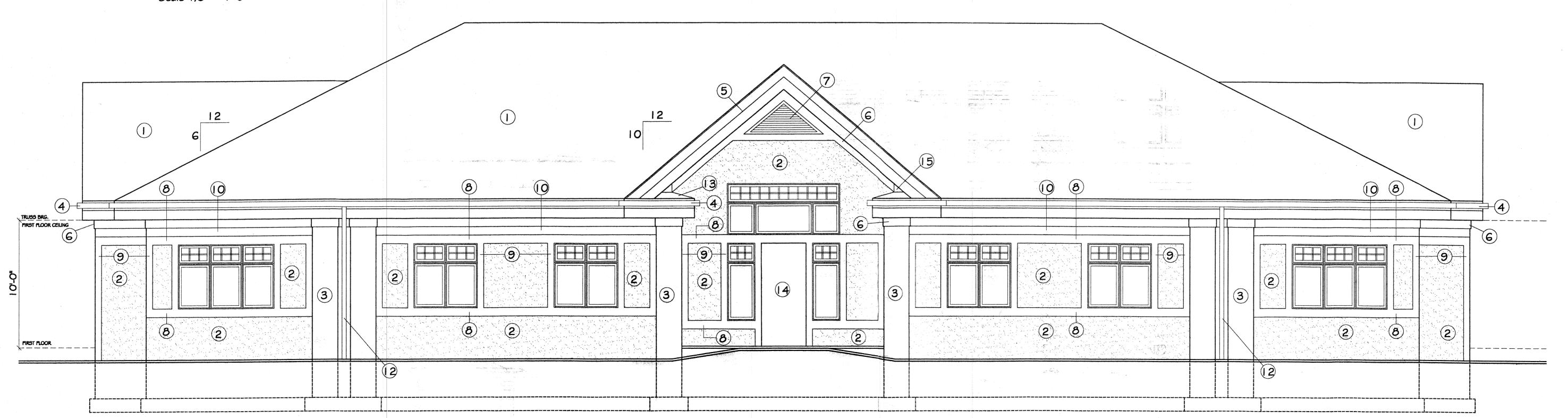
WEST ELEVATION

Scale 1/8" = 1'-0"

#### **ELEVATION LEGEND**

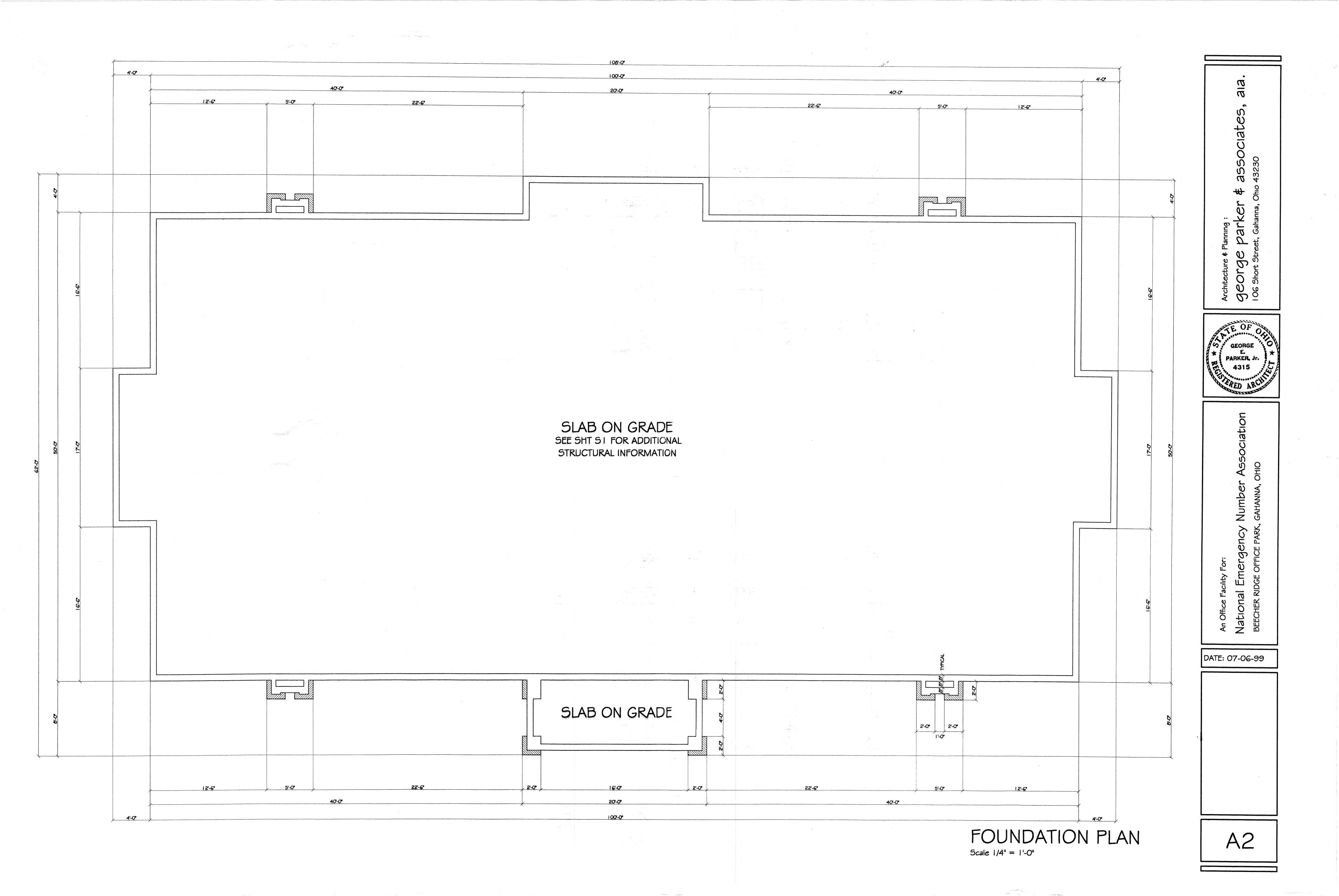
- (I) ASPHALT SHINGLES
- 2 STUCCO
- 3 STONE
- 4 ALUM. GUTTER OVER 1X WOOD FASCIAS-18" EXPOSURE
- (5) IX WOOD FASCIAS-18" EXPOSURE
- 6 5/4 X 6 FRIEZE
- 7 WOOD LOUVER

- 8 5/4 X 8 WOOD TRIM/FRIEZE
- 9 5/4 X 6 WOOD TRIM
- (19) 3-5/4 X WOOD CORNICE
- (I) TRIM FROM 5/4 MAT'L.
- 12 DOWNSPOUTS
- 13 SINGLE PLY RUBBER ROOFING
- SEE DOOR SCHEDULE & DIAGRAM
  ON SHT A 5
- 15 IX WD. TRIM AS REQUIRED



EAST ELEVATION

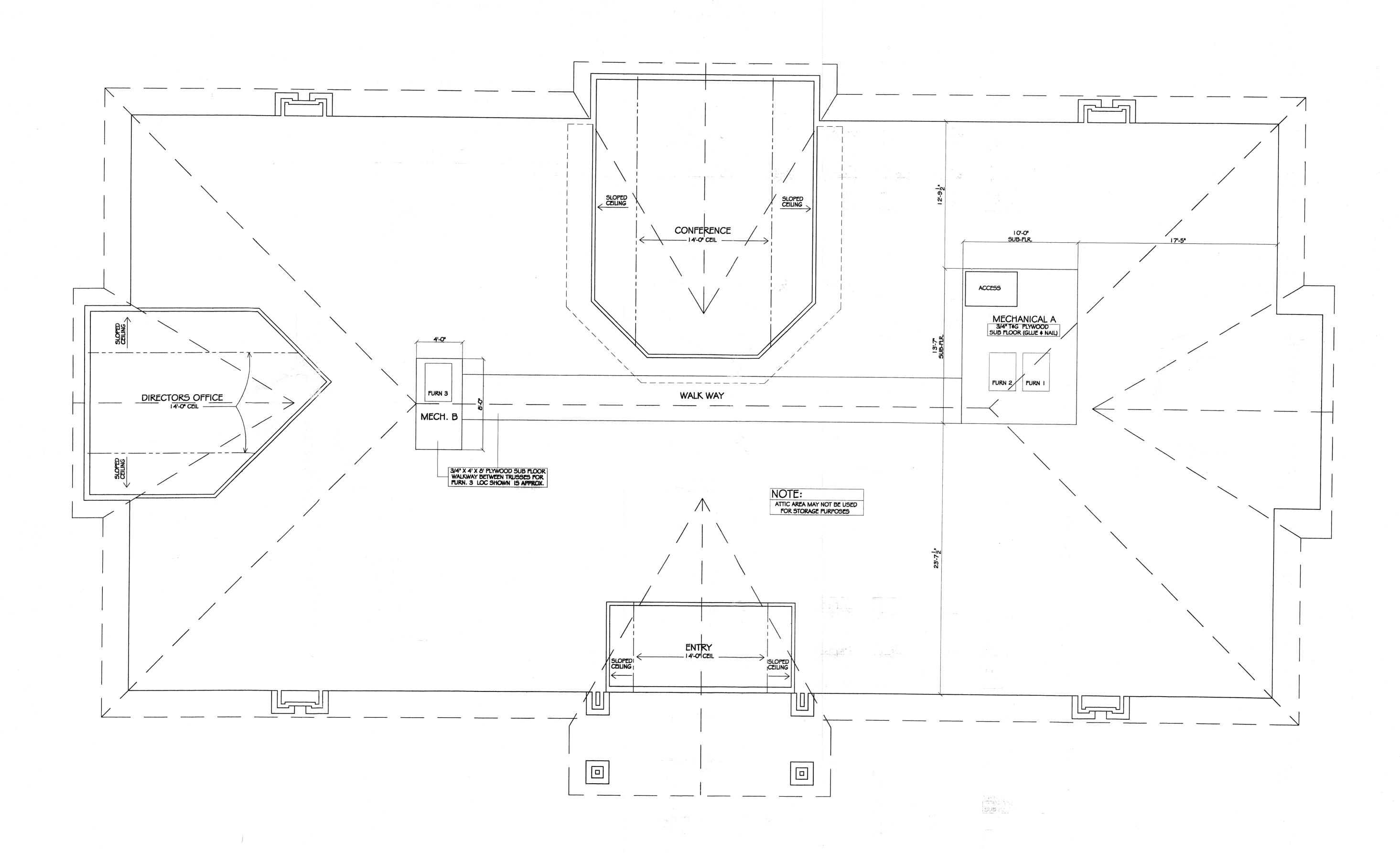
Scale 1/4" = 1'-0"

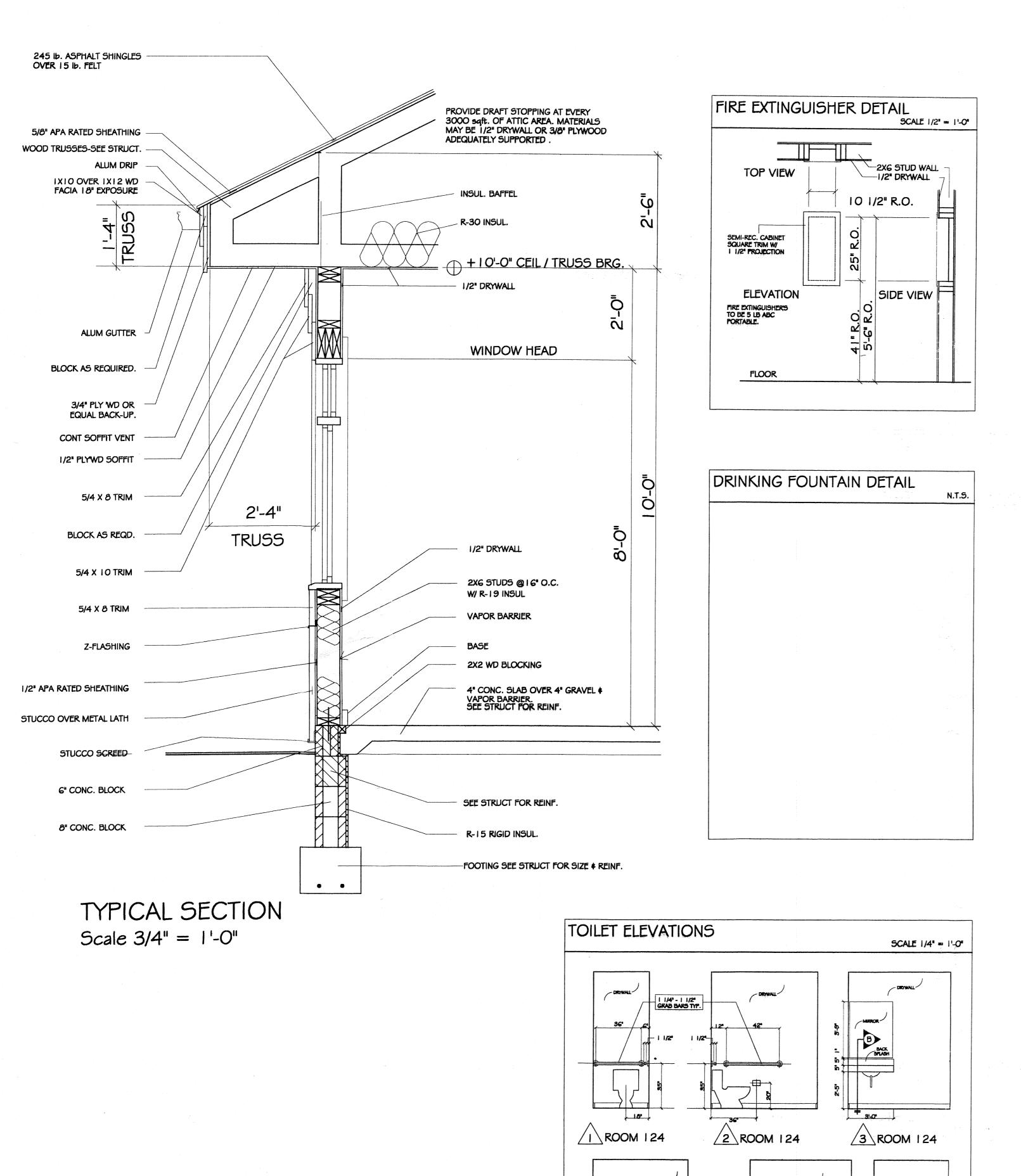


A4

ATTIC FLOOR PLAN

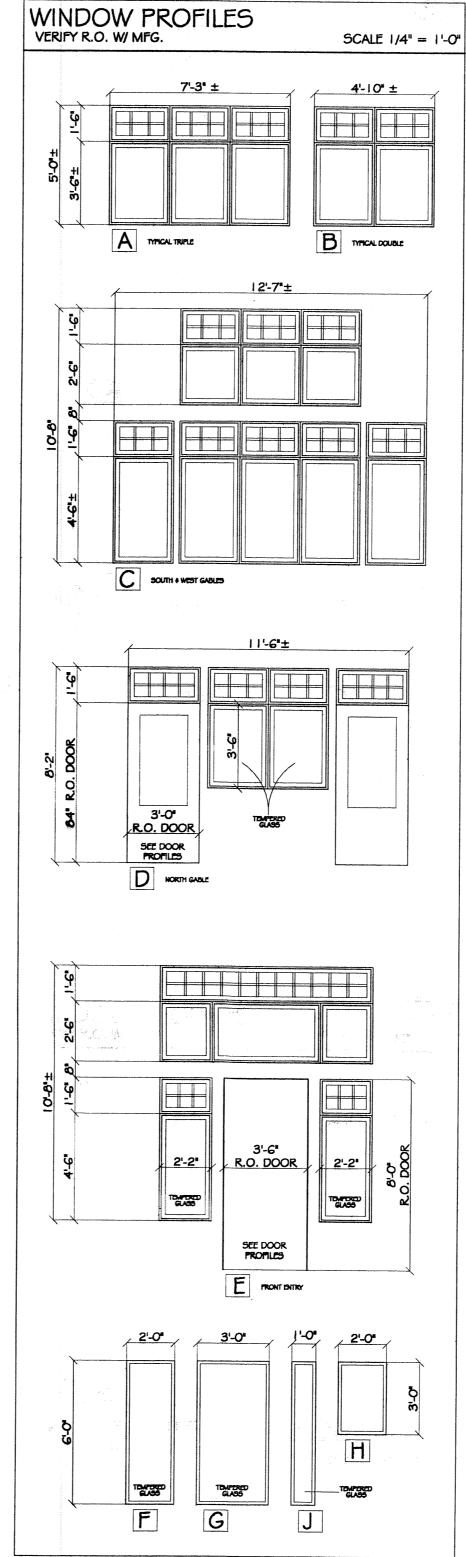
Scale 1/4" = 1'-0"





W4 8-19

4 ROOM 123



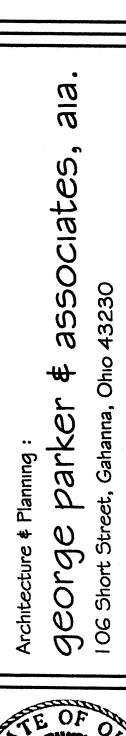
KEYED NOTES
BUILT IN CABINETS - GOORD W/OWNER
2 (SHADED AREA) SOFFIT AT 8'-0"
3 ROD & SHELF TYP. IN CLOSETS
4 COORD KITCH LAYOUT & CABS W/OWNER
5 FIRE EXTINGUISHER LOCATION. SEE DET SHT A5
6 EXIT SIGNAGE 🖾
7 DRINKING FOUNTAIN LOCATION. SEE DET SHT A5
8 TABLE 48" X 72" W/ ROUNDED END. COORD W/ OWNER
9 43" HIGH WALL TO REC. CAB & COUNTER
FOLDING STAIRWAY #444 HUSKY (ET 10) 22" X 54" R.O.

B SECTION B

5 ROOM 123

NO	NAME	FLOOR	BASE	WALLS	CEILING	CEIL HT	REMARKS
101	ENTRY	TILE	RUBBER	DRYWALL	DRYWALL	14'-0"	KLIVINIO
102	WAITING	TILE	RUBBER	DRYWALL	DRYWALL	10'-0"	
103	GATHERING	CPT	RUBBER	DRYWALL	DRYWALL	10'-0"	
104	RECEPTION	CPT	RUBBER	DRYWALL	DRYWALL	10'-0"	
105	ACCOUNTING	CPT	RUBBER	DRYWALL	DRYWALL	10'-0"	
106	CLOSET	CPT	RUBBER	DRYWALL	DRYWALL	10-0"	
107	DIRECTOR OF OPS.	CPT	RUBBER	DRYWALL DRYWALL		10-0"	
108	DIRECTORS OFFICE	CPT	RUBBER	DRYWALL DRYWALL		14'-0"	
109	CORRIDOR	CPT	RUBBER	DRYWALL	DRYWALL	10'-0"	
110	OFFICE I	CPT	RUBBER	DRYWALL	DRYWALL	10'-0"	
111	ASSISTANT	CPT	RUBBER	DRYWALL	DRYWALL	10-0"	<del></del>
112	DATABASE	CPT	RUBBER	DRYWALL	DRYWALL	10-0"	
113	CONFERENCE	CPT	RUBBER	DRYWALL	DRYWALL	14'-0"	
114	BREAK ROOM	VINYL	RUBBER	DRYWALL	DRYWALL	10-0"	
115	OFFICE 2	CPT	RUBBER	DRYWALL	DRYWALL	10'-0"	
116	OFFICE 3	CPT	RUBBER	DRYWALL	DRYWALL DRYWALL		
117	CORRIDOR	CPT	RUBBER	DRYWALL	DRYWALL	10'-0"	
118	WORK ROOM	CPT	RUBBER	DRYWALL	DRYWALL	10'-0"	
119	OFFICE 4	CPT	RUBBER	DRYWALL	DRYWALL	10-0"	
120	CONF. MANAGER	CPT	RUBBER	DRYWALL	DRYWALL	10-0"	
121	PUB. MANAGER	CPT	RUBBER	DRYWALL	DRYWALL	10-0"	
122	MEMBERSHIP	CPT	RUBBER	DRYWALL	DRYWALL	10-0"	
123	MEN HDC.	TILE	RUBBER	DRYWALL	DRYWALL	10'-0"	
124	WOMEN HDC.	TILE	RUBBER	DRYWALL	DRYWALL	10'-0"	

DOOR							FRAME		T	HWDE		
NO	TYPE	w	н	Т	MAT	FIN	MAT	FIN	THRSLD	GROUP	REMARKS	
1	A	42"	96"	1 3/4"	WOOD/ GLASS	PAINT	METAL	PAINT				
2	A	42"	96"	1 3/4"	GLASS WCCD/ GLASS	PAINT	<del> </del>	PAINT			-	
3	В	PR 36'	84"	1	GL/58   WD	PAINT	WD			2	-	
	В	36"	84"	1 3/8	WD	<del> </del>	WD	PAINT		3	BI-PASS	
4	В	36"	84"	1 3/8"	WD	PAINT	WD	PAINT		2		
5 6	В	36"	84"	1 3/8"	WD	PAINT	WD	PAINT		2		
7	В	36"	84"	<del>                                     </del>	WD	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	<del> </del>			2		
8	В	36"	<del>                                     </del>	1 3/8"	WD	PAINT	WD	PAINT		2		
9	В	PR 36'	84"	1 3/8"	WD	PAINT	WD	PAINT		2		
10	В	36"	84"	1 3/8"	WD	PAINT	WD	PAINT		2		
11	В	36"	84"	1 3/6"	WD	<del> </del>	1117			2		
12	В	36"	84"	1 3/8"	WD	PAINT	WD	PAINT		2		
13	В	<del> </del>		1 3/8"	WD	PAINT	WD	PAINT		2		
	В	36" PR 36"	84"	1 3/8"	WD	PAINT		PAINT		2		
14	С	36"	84" 84"	1 3/8"			WD	PAINT		2	<u> </u>	
15 A	C	36"	84"	1 3/4"	WOOD/ GLASS WOOD/	PAINT	METAL	PAINT		<u> </u>	<u> </u>	
		36"		1 3/4"	GLASS	PAINT	METAL	PAINT		·	FIXED	
16	В		84"	1 3/8"	WD	PAINT	WD	PAINT		2	ļ	
17	В	36"	84"	1 3/8"	WD	PAINT	WD	PAINT		2		
18	В	36"	84"	1 3/8"	WD	PAINT	WD	PAINT		2		
20	В	36"	84"	1 3/8"	WD	PAINT	WD	PAINT		4		
20	В	36"	84"	1 3/8"	WD	PAINT	WD	PAINT		4		
DC	OR	PR(	OFIL	ES,	/ TY	PE			HARI	DWAF	RE SCH.	
-	A	EXTERIOR / IN	OYER		B	) INTERIOR	R DOOR		GROUP			
TEMPERED GLASS PLUSH DOOR  RAISED RAISED									I 1/2 PAIR BUTTS, CLOSURE, LEVER PASSAGE SE SINGLE CYL. DEAD BOLT. GROUP 2			
PANEL 36°									I 1/2 PAIR BUTTS, LEVER PASSAGE SET			
EXTERIOR DOOR								GROUP 3				
			тем	if. Glass					PULL: PER I	5 ¢ TRACK MFG.	<b>(</b>	
	7.0							-	GROUP 4	4		
	1	1	ı					İ				



GEORGE

PARKER, Jr.

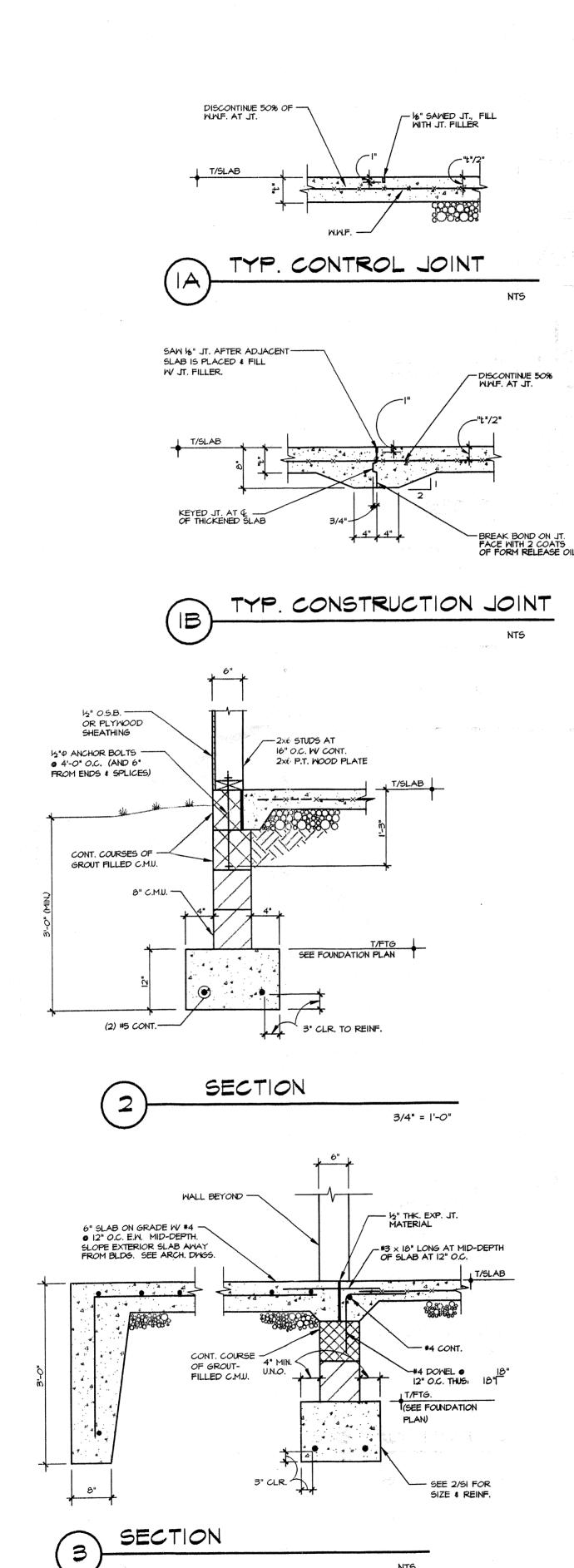
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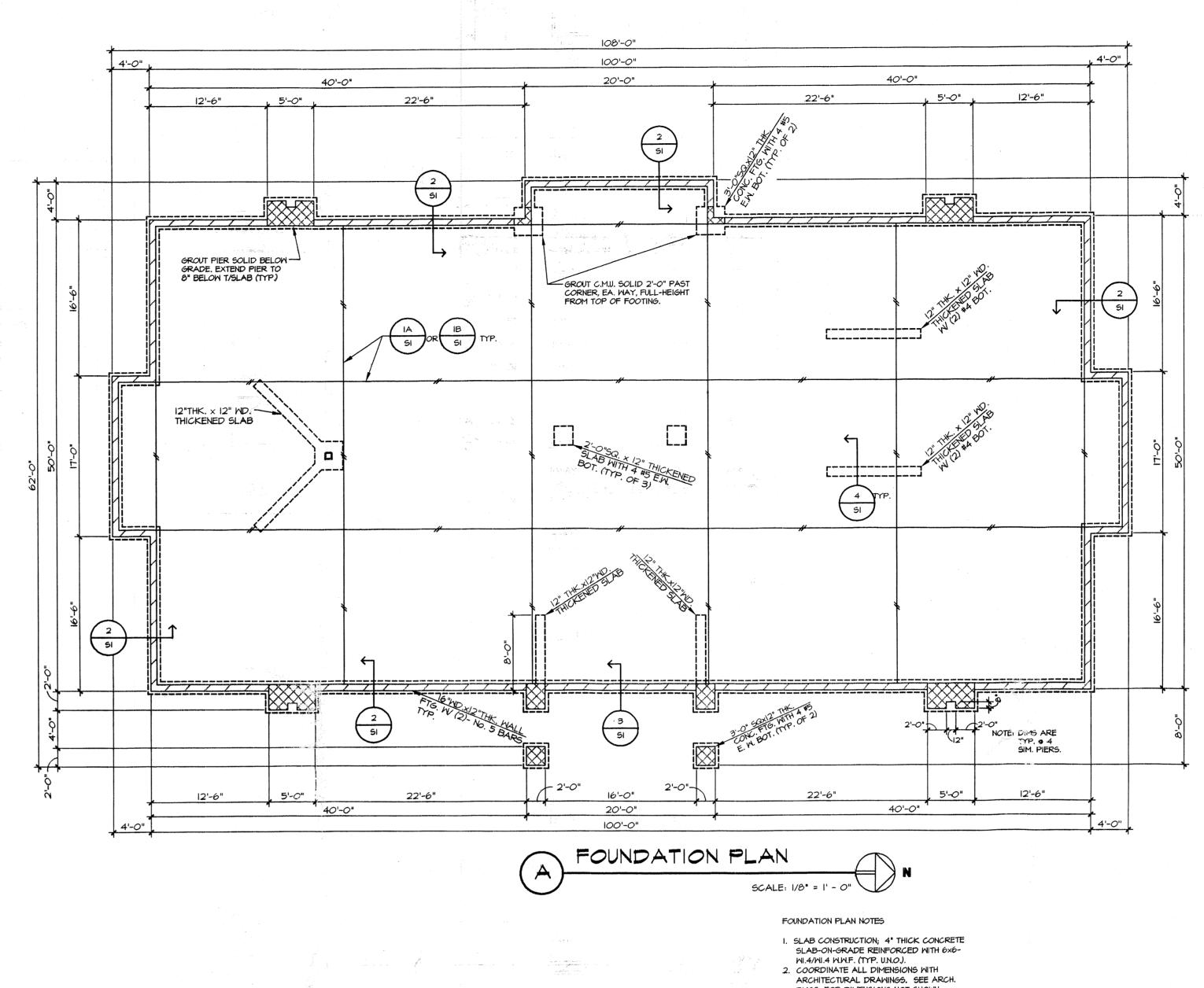
FRED ARCHI

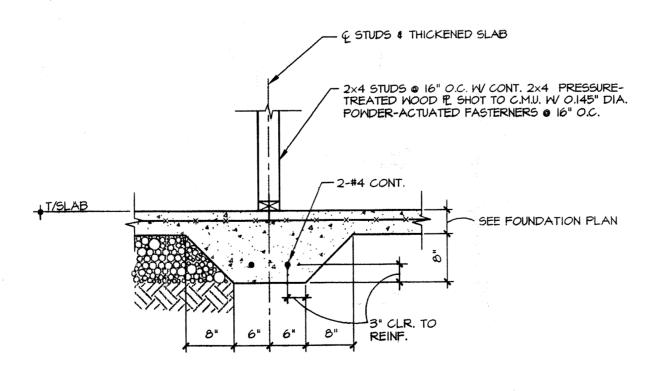
National Emergency Number Association BEECHER RIDGE OFFICE PARK, GAHANNA, OHIO

DATE: 07-06-99

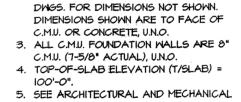
A5







SECTION N.T.S.



SLOPES TO DRAWINGS.

6. TOP OF FOOTING ELEVATION (T/FTG) = 97'- 4" TYP. U.N.O.
7. TOP OF PIER ELEVATION (T/PIER) = 99'- 4" T.Y.P. U.N.O.
8. UNLESS DIMENSIONED OTHERWISE, CENTER ALL FOOTINGS BELOW FOUNDATION WALLS.

9. SEE SHEET 52 FOR STRUCTURAL NOTES.

DRAWINGS FOR MINOR DEPRESSIONS AND

DATE: 07-06-99

Numb

SHIRK ENGINEERING, INC.
960 KINGSMILL PARKWAY, SUITE 201
COLUMBUS, OHIO 43229
TEL (614) 436-6465
FAX (614) 436-6485

51

I. THE STRUCTURE IS DESIGNED TO BE SELF-SUPPORTING AND STABLE AFTER THE BUILDING IS FULLY COMPLETED. IT IS SOLELY THE CONTRACTOR'S RESPONSIBILITY TO DETERMINE ERECTION PROCEDURE AND SEQUENCE TO ENSURE THE SAFETY OF THE BUILDING AND ITS COMPONENT PARTS DURING ERECTION, THIS INCLUDES THE ADDITION OF ANY SHORING, SHEETING, TEMPORARY BRACING, GUYS OR TIEDOWNS WHICH MIGHT BE NECESSARY. SUCH MATERIAL SHALL REMAIN THE CONTRACTOR'S PROPERTY AFTER THE COMPLETION OF THE PROJECT.

2. IT IS SOLELY THE CONTRACTOR'S RESPONSIBILITY TO FOLLOW ALL APPLICABLE SAFETY CODES AND REGULATIONS DURING ALL PHASES OF CONSTRUCTION.

3. EQUIPMENT FRAMING, LOADS, OPENINGS AND STRUCTURE IN ANY WAY RELATED TO HVAC, PLUMBING, OR ELECTRICAL REQUIREMENTS ARE SHOWN FOR BIDDING PURPOSES ONLY. CONTRACTOR SHALL OBTAIN APPROVAL OF THE INVOLVED TRADES BEFORE PROCEEDING WITH SUCH PORTION OF THE WORK. EXCESS COST RELATED TO VARIATION IN THESE REQUIREMENTS TO BE BORNE BY THE APPROPRIATE CONTRACTOR.

4. SHOULD ANY OF THE DETAILED INSTRUCTIONS SHOWN ON THE PLANS CONFLICT WITH THESE STRUCTURAL NOTES, THE SPECIFICATIONS, OR WITH EACH OTHER, THE STRICTEST PROVISION SHALL GOVERN.

5. GOVERNING CODE: 1998 OHIO BASIC BUILDING CODE.

6. DESIGN CRITERIA: a. ROOF LOADING:

I. DESIGN ROOF LIVE LOAD 25 PSF MINIMUM, PLUS THE EFFECTS OF SNOW DRIFTING

2. ROOF SHOW LOADS! a. GROUND SNOW LOAD b. FLAT-ROOF SHOW LOAD 18 PSF c. SNOW EXPOSURE FACTOR 0.7

d. SNOW IMPORTANCE FACTOR b. WIND LOADING:

3. EXPOSURE GROUP:

I. BASIC WIND SPEED 2. WIND IMPORTANCE FACTOR 3. WIND EXPOSURE CATEGORY: a. MAIN WIND FORCE RESISTING SYSTEM B b. COMPONENTS AND CLADDING 4. WIND DESIGN PRESSURES a. MAIN WIND FORCE RESISTING SYSTEM 20 PSF

b. COMPONENTS AND CLADDING c. SEISMIC DESIGN CRITERIA: I. EFFECTIVE PEAK VELOCITY: AV = 0.017 2. EFFECTIVE PEAK ACCELERATION: Aa = 0.10

4. PERFORMANCE CATEGORY: 5. SITE COEFFICIENT: 6. STRUCTURAL SYSTEM: LOADBEARING WALL SYSTEM 7. SEISMIC-RESISTING SYSTEM: LIGHT-FRAMED WALLS WITH SHEAR PANELS a. RESPONSE MODIFICATION FACTOR: R = 6-1/2

b. DEFLECTION AMPLIFICATION FACTOR: Cd = 4 6. ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE PROCEDURE 7. SHOP DRAWINGS

a. THE CONTRACTOR SHALL SUBMIT THREE COPIES OF THE FOLLOWING SHOP DRAWINGS TO THE ARCHITECT FOR REVIEW PRIOR TO FABRICATION (ONE COPY TO BE RETAINED BY THE ARCHITECT): I. CONCRETE REINFORCING

b. THE CONTRACTOR SHALL REVIEW AND ACCEPT FULL RESPONSIBILITY FOR DIMENSIONAL CORRECTNESS. ALL SHOP DRAWINGS SHALL BEAR THE APPROVAL STAMP OF THE CONTRACTOR.

I. THE CONTRACTOR SHALL BECOME FAMILIAR WITH THE SURVEY AND THE SUB-SURFACE INVESTIGATION REPORT BEFORE BEGINNING CONSTRUCTION.

2. NOTIFY THE ARCHITECT AS SOON AS POSSIBLE OF ANY UNUSUAL SOIL CONDITIONS OR SOIL CONDITIONS IN VARIANCE WITH TEST BORINGS, SUCH AS UNEXPECTED SPRING OR SEEPAGE WATER, MATERIAL DIFFERING FROM TEST BORINGS, OR SOIL OF QUESTIONABLE BEARING CAPACITY.

3. ALL FOOTINGS SHALL BEAR ON FIRM UNDISTURBED SOIL OR ENGINEERED FILL WITH AN ALLOWABLE BEARING CAPACITY OF 2000 POUNDS PER SQUARE FOOT. 4. PERIMETER FOOTINGS SHALL BEAR A MINIMUM OF 3'-O" BELOW EXTERIOR GRADE; STEP

FOOTINGS AS REQUIRED. WHERE UNDERGROUND UTILITIES ENTER OR EXIT THE BUILDING, STEP TOPS OF ALL FOOTINGS BELOW UTILITY INVERT ELEVATIONS. 5. BACKFILL AND FILL MATERIALS: PROVIDE SATISFACTORY SOIL MATERIALS FOR

BACKFILL AND FILL, FREE OF CLAY, DEBRIS, WASTE, FROZEN MATERIALS, VEGETABLE

6. EXCAVATION AND COMPACTION:

a. IT SHALL BE THE OWNER'S RESPONSIBILITY TO RETAIN THE SERVICES OF A SOILS ENGINEER TO INSPECT AND APPROVE FOUNDATION EXCAVATIONS FOR THE BEARING CAPACITY INDICATED ABOVE. THE CONTRACTOR SHALL COORDINATE WITH THE OWNER, THE SCHEDULING OF THE SOILS ENGINEER'S SERVICES WITH THE ANTICIPATED DATE OF CONCRETE PLACEMENT. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL FOUNDATION MODIFICATIONS AS RECOMMENDED BY THE OWNER'S SOILS ENGINEER.

b. KEEP FOUNDATION EXCAVATIONS FREE OF WATER AT ALL TIMES. REPLACE SOFT OR WEAKENED SOIL WITH LEAN CONCRETE (CLASS IV). c. THE EXISTENCE OF UNDERGROUND STRUCTURES AND/OR UTILITIES IS NOT KNOWN.

IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO COORDINATE WITH THE OWNER TO DETERMINE THE LOCATIONS OF ALL EXISTING UNDERGROUND STRUCTURES AND/OR UTILITIES. USE EXTREME CARE WHEN EXCAVATING SO AS NOT TO DISTURB ANY EXISTING UNDERGROUND STRUCTURES AND/OR UTILITIES. d. BACKFILL AND FILL SHALL BE PLACED IN LIFTS OF 8" MAXIMUM LOOSE DEPTH. EACH LIFT SHALL BE COMPACTED WITH A POWER VIBRATING COMPACTOR OR SIMILAR

EQUIPMENT TO ENSURE MAXIMUM COMPACTION OF THE MATERIAL. e. COMPACTION SHALL BE NOT LESS THAN 100% OF MAXIMUM DENSITY FOR COHESIVE OR COHESIONLESS MATERIAL, ACCORDING TO ASTM D698. DRAINAGE FILL AGAINST WALLS SHALL BE COMPACTED TO 96% OF MAXIMUM DENSITY.

7. WHERE LOOSE FILL MATERIALS ARE ENCOUNTERED, THE LOOSE MATERIALS SHALL BE OVER-EXCAVATED DOWN TO SUITABLE SOILS. THE OVER-EXCAVATED AREA SHALL THEN BE PROOF ROLLED AND FILLED WITH SATISFACTORY SOIL MATERIALS WHICH WILL PRODUCE THE RESULTS OF COMPACTION AND LOAD CARRYING CAPACITY REQUIRED. THE FILL SHALL BE PLACED AND COMPACTED IN ACCORDANCE WITH THE ABOVE PARAGRAPHS.

C. REINFORCED CONCRETE

I. ALL CONCRETE WORK SHALL BE IN ACCORDANCE WITH ACI 318-95, "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE."

2. SPECIFICATIONS: IN GENERAL, COMPLY WITH ACI 301-96, "STANDARD SPECIFICATION FOR STRUCTURAL CONCRETE."

3. ALL REINFORCING DETAILS SHALL CONFORM TO ACI 315-86, "MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES" UNLESS DETAILED OTHERWISE ON THE STRUCTURAL DRAWINGS.

4. MATERIALS: a. STRUCTURAL CONCRETE:

INTERIOR SLABS ON GRADE AND ALL INTERIOR CONCRETE NOT OTHERWISE IDENTIFIED EXTERIOR SLABS ON GRADE AND ALL EXTERIOR CONCRETE NOT (with air)

BACKFILL BELOW FOOTINGS

b. ALL DEFORMED REINFORCING BARS: ASTM A615, GRADE 60. C. ALL WELDED WIRE FABRIC: ASTM A185, DELIVERED IN FLAT SHEETS.

5. FIELD MANUAL: PROVIDE AT LEAST ONE COPY OF THE ACI FIELD REFERENCE MANUAL, SP-15, IN THE FIELD OFFICE AT ALL TIMES.

6. CONSTRUCTION JOINTS:

a. CONSTRUCTION JOINTS PERMITTED ONLY WHERE SHOWN OR AS APPROVED BY THE STRUCTURAL ENGINEER. ALL CONSTRUCTION JOINTS ARE TO BE KEYED. b. KEYED CONSTRUCTION JOINTS ARE REQUIRED IN ALL WALLS, WHETHER SHOWN OR NOT, AT MAXIMUM INTERVALS OF 40 FEET, UNLESS SHOWN OR NOTED OTHERWISE. ALL JOINTS SHALL BE KEYED WITH 2 x 4 CONTINUOUS KEYS UNLESS SHOWN

7. CONCRETE COVER: UNLESS NOTED OTHERWISE, DETAIL REINFORCING TO PROVIDE MINIMUM CONCRETE COVER AS FOLLOWS: a. CONCRETE CAST AGAINST AND PERMANENTLY

EXPOSED TO EARTH: 3 INCHES b. CONCRETE EXPOSED TO EARTH OR WEATHER! #5 BARS AND SMALLER 1-1/2 INCHES OTHERS 2 INCHES

D. MASONRY

I. ALL MASONRY WORK SHALL BE IN ACCORDANCE WITH THE FOLLOWING REFERENCES AND

a. AMERICAN CONCRETE INSTITUTE, COMMITTEE 530. b. NATIONAL CONCRETE MASONRY ASSOCIATION.

a. CONCRETE BLOCK: ASTM C90. MINIMUM NET AREA COMPRESSIVE STRENGTH OF CONCRETE MASONRY UNITS = 2,500 PSI. b. MORTAR: TYPE 5, MINIMUM COMPRESSIVE STRENGTH: 1,800 PSI.

C. BOND BEAM AND CORE FILL: ASTM C476, COARSE TYPE, MINIMUM COMPRESSIVE STRENGTH = 3,000 PSI. d. JOINT REINFORCING: MILL GALVANIZED FINISH, 9 GAGE MINIMUM SIDE WIRES AND CROSS WIRES. e. BAR REINFORCING: ASTM A615, GRADE 60.

3. MISCELLANEOUS;

a. VERTICAL COLLAR JOINTS TO BE FILLED SOLID WITH MORTAR. b. FILL CORE SOLID AROUND ANCHOR BOLTS.

c. HOLLOW MASONRY UNITS TO BE LAID WITH FULL MORTAR COVERAGE ON HORIZONTAL AND VERTICAL FACE SHELLS. WEBS SHALL ALSO BE BEDDED IN ALL COURSES OF PIERS, AND IN THE STARTING COURSE ON FOOTINGS, AND WHEN ADJACENT TO CELLS OR CAVITIES TO BE REINFORCED OR FILLED WITH CONCRETE OR GROUT. SOLID UNITS TO BE LAID WITH FULL HEAD AND BED

E. STRUCTURAL LUMBER

c. PLYWOOD:

a. STUDS: SPRUCE-PINE-FIR, STUD GRADE OR BETTER, ACCORDING TO THE NATIONAL LUMBER GRADES AUTHORITY (NLGA), SEASONED AT 1986 M.C. b. STRUCTURAL LUMBER: SPRUCE-PINE-FIR NO. 2 OR BETTER, ACCORDING TO THE NATIONAL LUMBER GRADES ASSOCIATION (NLGA), SEASONED AT 1996 M.C.

I. ROOF: C-C PLUGGED, 5/8" APA STRUCTURAL I RATED SHEATHING, 24/16, EXPOSURE I, U.N.O. 2. WALL: 1/2" APA STRUCTURAL I RATED SHEATHING, 24/16 O.C. EXPOSURE I.

2. CONNECTIONS: UNLESS NOTED OTHERWISE, IT IS THE CONTRACTOR'S RESPONSIBILITY TO PROVIDE PROPERLY DESIGNED CONNECTORS FOR THE END SUPPORT OF ALL MEMBERS. AS A MINIMUM, CONNECTIONS FOR STRUCTURAL MEMBERS SHALL CONFORM TO THE FASTENER SCHEDULE LISTED IN THE OHIO BASIC BUILDING CODE. a. JOISTS TO BEAMS: 16 GA. GALVANIZED STD. JOIST HANGERS BY THE SIMPSON

STRONG-TIE CO. (PROVIDE SLOPED AND/OR SKEWED HANGERS WHERE REQUIRED). b. PLYWOOD TO CEILING JOISTS: GLUED AND NAILED, USE 8d RING SHANK NAILS AT 6 INCHES O.C. AT PANEL EDGES AND 12 INCHES O.C. AT INTERMEDIATE c. PLYWOOD TO ROOF RAFTERS/TRUSSES: NAILED, USE 8d RING SHANK NAILS AT 6

INCHES O.C. AT PANEL EDGES AND 12 INCHES O.C. AT INTERMEDIATE SUPPORTS. PROVIDE PLYMOOD CLIPS AT MID-SPAN OF PLYMOOD BETWEEN SUPPORTS. d. PLYMOOD TO STUDS: USE 8d RING SHANK NAILS AT 6 INCHES O.C. AT PANEL EDGES AND 12 INCHES O.C. AT INTERMEDIATE SUPPORTS. e. SECURELY FASTEN ROOF RAFTERS TO CEILING JOISTS OR COLLAR TIES TO PREVENT LATERAL THRUST AT BASE OF RAFTER.

F. MULTIPLE STUDS SHALL BE NAILED WITH 8d NAILS AT 8" O.C., FULL LENGTH. g. PROVIDE POST CAPS AND BASES BY THE SIMPSON STRONG-TIE CO. AT TOPS AND BOTTOMS OF WOOD POSTS. h. PROVIDE 1/2" DIAMETER THRU-BOLTS FOR FLITCH PLATE BEAMS AT 24" O.C., STAGGERED TOP AND BOTTOM. LOCATE CENTERLINE OF BOLTS 2" FROM EDGE OF

LUMBER, TOP AND BOTTOM.

a. USE ONE LINE OF SOLID BLOCKING OR CROSS BRIDGING AT 8'-O" O.C. MAX. FOR ALL JOISTS AND RAFTERS. USE SOLID BLOCKING AT JOIST AND RAFTER BEARING LOCATIONS. PROVIDE SOLID BLOCKING BETWEEN JOISTS BELOW BEARING STUDS. b. USE SOLID BLOCKING AT MID-HEIGHT UP TO 5'-O" ON CENTER MAXIMUM, FOR ALL EXTERIOR STUD WALLS AND INTERIOR BEARING WALLS.

c. PROVIDE TRIPLE STUDS AT CORNERS, DOUBLE STUDS EACH END UNDER BEAM BEARINGS, AND DOUBLE STUDS WITH ONE KING STUD FACH END UNDER HEADER BEARINGS, UNLESS SHOWN OTHERWISE. d. PROVIDE A SINGLE PLATE AT THE BOTTOM AND A DOUBLE PLATE AT THE TOP OF

ALL STUD WALLS. SILL PLATES SHALL BE BOLTED TO FOUNDATION WALLS WITH 1/2" DIAMETER ANCHOR BOLTS AT A MAXIMUM OF 4'-0" O.C. AND 6 INCHES FROM ENDS AND SPLICES, UNLESS NOTED OTHERWISE. e. UNLESS NOTED OTHERWISE, PROVIDE DOUBLE 2 X 10 HEADERS OVER OPENINGS IN 2 X 4 STUD WALLS AND TRIPLE 2 X 8 HEADERS OVER OPENINGS IN 2 X 6 STUD

F. PROVIDE ONE LAYER OF 1/2" THICK PLYWOOD BETWEEN EACH MEMBER OF DIMENSIONAL LUMBER HEADERS. g. PRESSURE TREAT ALL EXTERIOR LUMBER AND ALL LUMBER IN CONTACT WITH CONCRETE OR MASONRY. h. PROVIDE PLYWOOD WALL SHEATHING ON ALL EXTERIOR WALLS.

I. PROVIDE AND INSTALL BRIDGING FOR PREFABRICATED WOOD TRUSSES AS INDICATED ON THE TRUSS MANUFACTURER'S APPROVED SHOP DRAWINGS. 4. LYL INDICATES LAMINATED VENEER LUMBER (MICRO-LAN MEMBER BY THE TRUS-JOIST CORP). CONNECT MULTIPLE MEMBERS AS FOLLOWS.

a. DOUBLE MEMBERS: NAILED, USE 2-16d NAILS AT 12" OC., LOCATED 2" FROM TOP AND BOTTOM OF BEAM. b. TRIPLE MEMBERS: BOLTED, USE 1/2" DIAMETER BOLTS AT 24" O.C., LOCATED 2" FROM TOP AND BOTTOM OF BEAM.

F. PREFABRICATED WOOD TRUSSES.

a. LUMBER: SPECIES PER DESIGN BY TRUSS MANUFACTURER; NO. 2 GRADE OR BETTER, EXCEPT STUD GRADE MAY BE USED FOR WEB HEMBERS; 15% MAXIMUM M.C. b. CONNECTIONS: ALL INTERNAL CONNECTIONS ARE TO BE DESIGNED BY THE TRUSS SUPPLIER. METAL CONNECTOR PLATES: GALVANIZED SHEET STEEL PER ASTM A653, COATING CLASS 660. MANUFACTURE WITH HOLES, PLUGS, TEETH, OR PRONGS UNIFORMLY SPACED AND FORHED. c. HANGERS: ALL TRUSS-TO-TRUSS HANGERS ARE TO BE DESIGNED AND PROVIDED BY

THE TRUSS SUPPLIER. a. ROOF LOADS:

TOP CHORD LIVE LOAD: 25 PSF TOP CHORD DEAD LOAD: 10 PSF BOTTOM CHORD LIVE LOAD: 20 PSF, PER OBBC BASED ON WEB CONFIGURATION, U.N.O. BOTTOM CHORD DEAD LOAD: 5 PSF NET WIND UPLIFT: 8 PSF

b. IN ADDITION TO THE UNIFORM LOADS INDICATED ABOVE, DESIGN TRUSSES FOR ALL SUPERIMPOSED DEAD LOADS INCLUDING BUT NOT LIMITED TO OVERLAY FRAMING, CHIMNEYS, MECHANICAL UNITS, ETC. DESIGN TRUSSES FOR THE EFFECTS OF SNOW DRIFTING WHERE APPLICABLE. b. DESIGN OF MEMBERS AND CONNECTIONS IS TO BE BY A PROFESSIONAL ENGINEER,

REGISTERED IN OHIO, EXPERIENCED IN SIMILAR DESIGN, RETAINED BY THE MANUFACTURER. c. DESIGN BOTTOM CHORD OF GIRDER TRUSSES FOR THE END REACTION OF SUPPORTED TRUSSES. THE DESIGN OF ALL HANGER CONNECTORS SHALL BE THE

RESPONSIBILITY OF THE TRUSS SUPPLIER

a. SUBMIT TRUSS SHOP DRAMINGS WHICH EXHIBIT THE SEAL OF THE ENGINEER RESPONSIBLE FOR THE TRUSS DESIGN. b. SUBMIT LAYOUT DRAWING WHICH INDICATES THE LOCATION OF EACH TRUSS.

c. SUBMIT HANGER CONNECTOR TYPES AND LOCATIONS.

> CONTRACTOR OPTION: OVERLAY TRUSSES NAILED DIRECTLY TO TOP CHORDS OF

> PRIMARY TRUSSES MAY BE USED IN LIEU

TOP CHORD OF PREFAB

ROOF TRUSSES -

2x6 RAFTERS -

SEE ARCH. DWGS,

CONT. VALLEY PLÁTE (FLAT).

NAIL (THROUGH SHEATHING)

TO TOP CHORDS W 3-8a

NAILS EA. TRUSS (TYP)

VARIES"

CONT. RIDGE— PLATE

OMIT SHEATHING OVER-

FRAMING ABOVE.

OF FIELD FRAMING SHOWN.

ARCH. DWGG. -5/8" PLYWOOD SHEATHING (TYP.) TRUSS BEARING 1/2" PLYWOOD OR 0.5.B. SHEATHING. EXTEND FULL-HEIGHT TO UNDERSIDE OF TOP CHORD SIMPSON H-I HURRICANE -

GIRDER TRUSSES, UNLESS NOTED OTHERWISE. BUT, IN ALL CASES, FROVIDE FULL BEARING WIDTH BELOW MULTIPLE-HEMBER BEAM, HEADER OR GIRDER TRUSS. 5. ALL HEADERS TO BE (3) 2 X 8, U.N.O. 6. ALL ROOF TRUSSES ARE TO BE SPACED AT MAXIMUM 24 INCHES O.C., UNLESS NOTED

2x6 STUDS AT 16" O.C. Y

2×4 SPLICED END BOARD-

CONNECT CONT. 2x4 SILL PL WITH -

T/PLYWOOD EL. IIO'-IO"

SEE PLAN FOR

FLOOR JOISTS

34" T&G PLYWOOD.

(2)#12 x 4" WOOD SCREWS @ 16" O.C.\

7. ELEVATION OF BOTTOM OF COLLAR TIES OVER ENTRY IS 14'-0".

ROOF FRAMING PLAN

. ROOF CONSTRUCTION: 5/8" O.S.B.

SHEATHING ON PRE-FABRICATED ROOF

CINTER, UNLESS NOTED OTHERWISE.

2. COORDINATE ALL DIMENSIONS WITH

FACE OF SHEATHING, U.N.O.

ARCHITECTURAL DRAWINGS. SEE

3. ALL EXTERIOR STUD WALLS ARE 2 X 6

4. PROVIDE MINIMUM DOUBLE STUD POST

UNDER EACH END OF ALL BEAMS AND

HEADERS AND PRE-FABRICATED WOOD

STUDS SPACED AT 16" ON CENTER, U.N.O.

TRUSSES SPACED AT MAXIMUM 2'-0" ON

ARCHITECTURAL DRAWINGS FOR DIMENSIONS

NOT SHOWN. DIMENSIONS SHOWN ARE TO

POS FRAMING PLAN NOTES

(2) EA. SIDE OF HIGH HEADER (BEAR ON

LOW HEADER)

(FULL HEIGHT)

(3)- 2x12 (HI)

, 3) EA. SIDE OF LOW HEADER

EXTENT OF

ACCESS TO

MALKWAY FOR

SEE ARCH. DWGS

(2) 134"×914" LVL + (1) 2×10 (HIGH), —

(3)-2×10

GIRDER TRUSS

PROVIDE 2x12

TOP CHORD

(3) 134"×914" LVL (LOW).

(3)-2×10

MECHANICAL-

ROOM IN

CONTRACTOR NOTE:

N LIEU OF POST AND BEAM

FRAMING SHOWN, TRUSS MFR.

MAY PROVIDE PRE-ENGINEERE

TRUSSES TO FRAME PORTIONS

WITH RAISED CEILINGS.

52

SIDE

SEE WEST -

WINDOWS FOR

SIM. HEADERS

# KING STUDS

ANCHOR, EACH END OF EACH TRUSS TYP.

TRUSS MFR. NOTE: DESIGN TOP CHORDS

2'-0" @ AREAS W OVERLAY FRAMING ABOVE.

OF TRUSSES FOR UNBRACED LENGTH=

PROVIDE 2x4 VERT.

RAFTERS (TYP)

STRUT FROM EVERY

THIRD TRUSS TO 2x6

RAFTERS. USE ALTERNATE

-5/8" OSB SHEATHING.

SEE STRUCTURAL NOTES

FOR NAILING REQUIREMENTS.

TRUSSES AT ADJACENT

- NOTE:

42x6 @ 2'-0"

TRUSS W (3) 12d

TOP CHORDS OF TRUSSES (FLAT), NAILED TO

@ AREAS W OVERLAY TOP CHORD OF EA

SECTION

AND JACK

X- INDICATES THE NUMBER OF 2x6 JACK STUDS - INDICATES THE NUMBER OF 2x6 KING STUDS

LAP 2x4 STUD

TO EACH TRUSS TO

NAIL WITH (2) IOd

NAILS @ 16" O.C.,

EACH TRUSS

-2x4 STUDS AT 16" O.C.

SUPPORT TRUSS ABOVE.

SEE TRUSS PROFILES.

- SIMPSON HI HURRICANE

ANCHOR, EACH. END OF

-(2) 2x10 BOND BOARD

Era Minipanyaya

TRUSSES • HIPS. PROVIDE 2X12 TOP CHORD WHERE FIELD-DESIGN BOTTOM CHORD OF GIRDER TRUSS FOR END REACTION FRAMED RAFTERS OF SUPPORTED TRUSSES 10 o Tla 10 @ Tla -OMIT TAIL @ INTERIOR CONDITIONS (TYP) SEE PLAN SEE ARCH. DWGS. -— SEE ARCH. DWGS FACE OF STUD --WALKWAY OPENING THROUGH TRUSS. COORD. DIMS. WITH ARCH. DWGS. TRUSS TI, TIa DESIGN BOT. CHORD. FOR 10 PSF DL + 25 PSF LL. WALKWAY OPENING

THROUGH TRUSS. COORD. DIMS. WITH ARCH. DWGS. DESIGN BOT. CHORD. FOR 10 PSF DL + 25 END OF TRUSS= PER TRUSS MFR. FACE OF SUPPORTING END OF TRUSS = -GIRDER TRUSS FACE OF SUPPORTING

TRUSS T3 GIRDER TRUSS 1/2×914 STL PL & 2-2×10 EA. SIDE W 15" O THRU BOLTS @ 24" O.C. STAGGERED, TYP. & SUPPORTING

- STEP TOP CHORD . HIPS. SEE ROOF FRAMING PLAN. OMIT TAIL . INTERIOR SEE ARCH. DWGS. -- END OF TRUSS = FACE OF SUPPORTING PER TRUSS MFR. GIRDER TRUSS OUTSIDE FACE OF -WALL SUPPORT

SIDE

) ) EA. SIDE

SEE B/S2 FOR

ATTIC FLOOR

BELOW

RAFTERS 6 16"

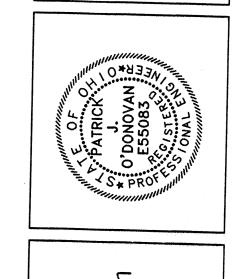
O.C. WITHIN SHADED REGION. SEE 3/S2 (TYP.).

FRAMING PLAN

SEE B/S2 FOR -ATTIC FLOOR FRAMING WORK THIS TRUS MITH TI SEE ARCH. DWGS. -SEE ARCH. DWGS FACE OF STUD -

FOR DIMS.

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